CITY OF MILLERSBURG, OREGON

for the construction of the

TRANSITION PARKWAY AND LINEAR PARK

VOLUME 5 REFERENCE INFORMATION

Project No. 2022-006

Bid Documents

JACOBS

Corvallis, Oregon

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CITY OF MILLERSBURG TRANSITION PARKWAY AND LINEAR PARK

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1. Radiation Sources and Remedial Actions for the SAA Property in Millersburg, OR	

Technical Memorandum

Radiation Sources and Remedial Actions for the SAA Property in Millersburg, OR

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Introduction

This memorandum addresses potential concerns regarding the naturally occurring radiological material (NORM) that is present at the Soil Amendment Area, or SAA (hereafter "Site"). The Site is located on Conser Road NE in Millersburg, Oregon (Linn County) and is approximately 41 acres in size. In 1975 and 1976, the Oregon Department of Environmental Quality (DEQ) issued a solid waste permit for a one-time application of solids from ATI's (formerly Teledyne Wah Chang) wastewater treatment system as an experimental soil amendment. The solids applied to the soil contained NORM from the processing of zircon sands. The Site was listed on the National Priorities List (NPL) in 1983 by the Environmental Protection Agency (EPA). Ownership of the SAA was transferred to the City of Millersburg in 1990 (EPA 2006).

Radiation Sources at the SAA

The NORM used on the SAA consisted primarily of radium-226. Radium is a naturally occurring, silvery white radioactive metal that is formed when uranium and thorium decay in the environment. It has been found at very low levels in soil, water, rocks, coal, plants, and food (NCRP 1984; WHO 2012). Radium-226 decays into radon-222 when it undergoes a nuclear transformation. The rate of this transformation is driven by the half-life of radium-226, or the time required for half of the radium-226 atoms to transform into radon-222. Given that the half-life of radium-226 is 1,602 years, the radiological concerns associated with its presence may require remedial actions and institutional controls for extensive periods of time.

Radon-222 is a gas that when inhaled represents one of the largest sources of radiation exposure in the environment and is found in both outdoor and indoor air of buildings of all kinds (EPA 2003). Radon-222, with a 3.8-day half-life, decays to into a series of short-lived decay products that get filtered by the human lungs when inhaled. The National Council on Radiation Protection and Measurements (NCRP) has documented contributions from sources of radiation exposure to the total radiological dose per individual in the U.S. population, most recently for the year 2006 (NCRP 2009). The NCRP report estimates that radon exposure accounts for ~75% of an individual's yearly dose from natural background sources (not including medical sources).

Radon levels vary widely across the United States, with elevated levels most commonly found in the Appalachians, the upper Midwest, and the Rocky Mountain states (NCRP 2009; Marsh et al. 2010). The average radon concentration in the indoor air of America's homes is about 1.3 pCi/liter (pico-Curie per liter of air). For enclosed structures in Linn County, the average radon concentration is less than 2.0 pCi/ liter. The average radon concentration in outdoor air is 0.4 pCi/ liter, or one-tenth of EPA's 4 pCi/ liter action level (EPA 2003). The EPA action level of 4 pCi/ liter is directly correlated to the amount of radium-226 present in the ground surrounding and beneath an enclosed structure. The presence of radium-226 is typically measured in units of pCi per gram of material, or pCi/gram.

Terrestrial radiation, or external terrestrial radiation, consists of radiation emitted from natural sources in the earth and accounts for roughly 7% of an individual's yearly background exposure (NCRP 2009). The primary path of exposure from terrestrial radiation is simply being in close proximity to the source in the environment (e.g., standing on soil, concrete, etc.). Sources such as radium-226 and the short-lived radon-222 decay products are contributors to terrestrial radiation due to the gamma-ray and x-ray emissions that are associated with their decay. Accurate measurement of the external gamma radiation levels in units of

µrem/hour (micro-rem per hour) is used to monitor and control potential exposures to terrestrial radiation sources.

Remedial Actions for SAA

There are two governing documents that provide the regulatory framework for the Site as it pertains to NORM and corresponding radiological safeguards and remedial actions:

- 1. The EPA Record of Decision, Declaration, Decision Summary and Responsiveness Summary of Final Remedial Action for Surface and Subsurface Soil Operable Unit (the "ROD") with ATI (EPA 1995).
- 2. The City of Millersburg Consent Decree (EPA et al. 2006).

In the ROD, dated September 27, 1995, the EPA Region 10 Regional Administrator selected a remedial action plan for the Site (EPA 1995).

The ROD remedial actions pertaining to radionuclide contamination addressed the risks posed by surface external gamma radiation and radon. From Section 10.1 of the ROD, it is stated that Site

Areas with surface gamma radiation levels exceeding 20 µrem/hour over background levels (equal to 30.5 µrem/hour) averaged over 100 square meters will be excavated.

The SAA uses a "reference level" of 12.5 μ rem/hour (ROD Section 6.4.1.1) as background (rather than the 10.5 μ rem/hour Main Plant background level), resulting in a gamma cleanup level of 32.5 μ rem/hour. From Table 6-5 and Figure 6-6 of the ROD, it is clear that none of the averaged gamma radiation levels exceeded the 32.5 μ rem/hour cleanup level (20 μ rem/hour above the reference level, measured at a height of 1 meter). In fact, none of the average levels were found to be more than 10 μ rem/hour over the reference level. As a result, no remedial action pertaining to external gamma radiation levels was required for the SAA.

In November of 2021, an SAA scoping survey was performed by GSI Water Solutions, Inc. on behalf of the City of Millersburg. The results of this survey were published in a technical memorandum on January 28, 2022 entitled *Soil Amendment Area Scoping Survey Summary*. The memorandum demonstrated that the geospatial behavior in 2021 is consistent with that of the 1995 ROD gamma survey. In their December 5, 2022 report entitled *Evaluation of SAA Remedial Actions and GSI Survey Results*, the Certified Health Physicists at Radian Scientific demonstrated that the results presented in the GSI memorandum are consistent with the conclusion of the 1995 ROD that no areas in the SAA exceed the gamma radiation levels of 20 µrem/hour above the reference level. In fact, their review suggested that the gamma radiation levels at the SAA are more than likely at or below the levels measured in the original 1995 ROD and therefore the ROD conclusion of no remedial actions pertaining to surface gamma radiation levels is still valid.

Remedial actions for mitigating radon in future buildings are provided in Section 10.2 of the ROD where it is stated that

Action for radon is required for the entire Soil Amendment Area, and for areas on the Main Plant plan where surface and subsurface soil radium-226 concentrations exceed 3 pCi/gram. These areas could exceed the action level for radon of 4 pCi/liter if buildings are constructed in the future.

Additionally,

The selected remedy requires that future buildings be constructed using radon controlling construction methods.

It goes on to say,

The only other effective remedial alternative for mitigation of radon in the Soil Amendment Area was excavation of soil to background levels. This option was eliminated as being prohibitively expensive. Current plans for the Soil Amendment Area are for use as an industrial park. During a meeting with the city of Millersburg, it was suggested that the contaminated material in the Soil Amendment Area might be excavated and used to construct landscaping and berms. The efficacy of this potential option has not been considered. However, if it is later offered as a potential option by the City, proves viable, and meets the remedy selection criteria, EPA may reconsider this portion of the selected remedy.

The EPA did in fact reconsider this alternative in the *Explanation of Significant Differences to the To the September 27, 1995 Record of Decision: Final Remedial Action for Surface and Subsurface Soil Operable Unit* (the "ESD"; EPA 2001), where it provided the city of Millersburg the option for radium-226 contaminated soil in the SAA to be excavated to an EPA approved radium-226 concentration with the excavated material being used in on-site berms or disposed of in an EPA approved off-site landfill (Section 4.6 of ESD; EPA 2001). The EPA approved radium-226 level is one that is "statistically indistinguishable" from background. The risk from radon in future buildings must not exceed an excess cancer risk of 1x10⁻⁴ (Section 4.2.2.3 of ESD; EPA 2001). Modeling results indicate that approximately 0.3 pCi/gram radium-226 equates to the 1x10⁻⁴ risk level. This concentration is approximately 25% of the background radium-226 concentration and is unlikely to be differentiable from background levels analytically.

In October of 1996, the City of Millersburg, the EPA, the Oregon Department of Environmental Quality (ODEQ) and Teledyne Wah Chang entered into a Consent Decree implementing the decisions outlined in the ROD (EPA 2006). Exhibit C of this Consent Decree is the *Requirements for Remedial Design and Remedial Action (RD/RA) for Soil Removing or Berming* and Exhibit A is the *Environmental Protection Easement and Equitable Servitude*. In Section 3 of Exhibit A, it is stated that the applicability of building construction requirements on the SAA "does not apply to open structures such as parking areas (including parking areas that are covered, but not enclosed), exterior storage areas, utility vaults, or other similar structures." Furthermore, the introductory paragraph of Exhibit C goes on to say that "earth moving and site grading activities associated with development or construction do not constitute excavation activities subject to this Exhibit C."

The Consent Decree specifically differentiates between excavation for purposes of building an enclosed building and the earth moving and site grading activities that may occur at the SAA for other purposes. According to the Consent Decree, the building of roads, parking lots, storage areas, etc., would not require any radon mitigation building techniques, nor would they require soil excavation to an EPA approved radium-226 level. Activities such as earth moving and site grading are not considered excavation and the materials moved within the SAA would not require berming or disposal at an EPA-approved landfill.

Radiological Risks Associated with SAA

Risk in the context of radiation exposure is the increased chance of developing cancer above the rate normally expected in the population at large, where according to the most recent report by National Academy of Sciences Biologic Effects of Ionizing Radiation (BEIR), 42 out of 100 persons are expected to develop cancer from sources other than radiation exposure in their lifetime (NRC 2006). Risk estimates that are used to predict public health effects are based on detailed epidemiological studies of exceedingly well-defined populations. Such studies have not demonstrated health effects to individuals exposed to less than 10,000 millirem (mrem) as statistical limitations making it difficult to evaluate cancer risk in humans at these lower doses (HPS 2004; NRC 2006). A comprehensive review of available biological and biophysical data led the BEIR committee to assume that the risk would continue in a linear fashion at lower doses without a threshold (i.e., any dose greater than zero has the potential to cause a small increase in risk to humans). This assumption is termed the "linear-no-threshold" (LNT) model.

The EPA performed a detailed radiological human risk assessment for the SAA, as documented in Section 7.0 of the ROD (EPA 1995). The EPA used an incremental excess lifetime cancer risk threshold of $1x10^{-4}$ (incremental means risk in excess of background risk). An incremental excess cancer risk of $1x10^{-4}$ indicates that an individual has a 1 in 10,000 chance of developing cancer when exposed to a given amount of radiation over their lifetime. The radiation dose corresponding to a risk level of $1x10^{-4}$ is 100 mrem (NRC 2006). While this dose is 100 times lower than the 10,000 mrem discussed previously, the LNT model provides a risk estimate associated with this low dose. By comparison, the background or normal lifetime risk of developing cancer is $4.2x10^{-1}$ (42 out of 100).

The threshold value of $20 \,\mu$ rem/hour (or $0.02 \,\text{mrem/hour}$) above background is selected to ensure the risk from gamma exposure is minimized. Exposure to the external gamma radiation field of the SAA occurs when an individual is either standing directly on the SAA soil surface or on a ground covering directly over the SAA soil (parking lot, road, slab foundation, etc.). The EPA considered the current use of the SAA (agricultural) and anticipated future use (industrial) when identifying likely exposure scenarios (EPA 1995). In the agricultural setting, the individual is assumed to spend 40 hours per week outside for a period of 30 days per year for 25 years. In the industrial setting, the individual is assumed to spend 10 hours per week outside and 30 hours per week inside a building located on top of SAA soil for a period of 250 days per year for 25 years. Based on the average gamma exposure rates of the SAA, the incremental excess lifetime cancer risk for the agricultural scenario was found to be of 9.1×10^{-6} (EPA 1995). The average incremental excess lifetime cancer risk for the future industrial scenario was 5.8×10^{-5} . Both of these scenarios are below the EPA threshold value of 1×10^{-4} and can be considered insignificant when compared to the normal cancer risk from sources other than radiation.

The 20 μ rem/hour above background threshold is consistent with the EPA's indoor gamma radiation limit for the remediation of inactive uranium processing sites (EPA 1982). The National Research Council estimated that the annual gamma radiation dose associated with 20 μ rem/hour above background is roughly 100 mrem (i.e., risk level of $1x10^{-4}$) (NRC 1999). As a point of reference, the NCRP estimates that on average, a person in the United States receives about 311 mrem of dose each year from natural background radiation sources (NCRP 2009).

Conclusion

The ROD, ESD, and Consent Decree provide very clear guidance on remedial actions required for the SAA. If an enclosed building is going to be built on the SAA, the risk of radon must be mitigated in one of two ways:

- 1. If no excavation of radium-226 contaminated soils will occur at the proposed build site, enclosed buildings must be constructed using radon controlling construction methods.
- 2. In the absence of approved radon controlling construction methods, radium-226 contaminated soils must be excavated to a cleanup level that is statistically indistinguishable from background. The excavated soil can be bermed at the SAA or disposed of at an off-site landfill (ESD Section 4.6.2).

Exhibit A and C of the Consent Decree state that such remediation actions are not required for activities such as road building, parking lot building, or earth moving and site grading.

The risk associated with external gamma radiation is mitigated by ensuring that radionuclides present in the SAA soils do not result in a gamma dose rate greater than 20 µrem/hour above background, measured at 1 m above the surface and averaged over 100 m². Areas where this cleanup level is exceeded must be excavated. It has been concluded in the ROD, and subsequently supported by analyzing the GSI scanning survey results, that the SAA does not contain any areas that need to be excavated to mitigate the gamma radiation risks.

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2. Geotechnical Data Report

Jacobs

Geotechnical Data Report

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City of Millersburg

Millersburg Transition Parkway Project Volume 1 May 20, 2024







Geotechnical Data Report

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Geotechnical Data Report

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Acronyms and Abbreviations

ASTM ASTM International

bgs below ground surface

City City of Millersburg

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1. Introduction

This Geotechnical Data Report provides a summary of geotechnical data collected for the Millersburg Transition Parkway Project for City of Millersburg (City). The City is going to construct improvements to the Central Industrial Property, located adjacent to and south of Conser Road NE between the existing segment of NE Transition Parkway and Woods Road NE to support industrial development. The project improvements include an arterial road, stormwater management facilities for the arterial road, extension of water and sewer utilities in the arterial road right of way, and a linear park including a multi-use path to provide a buffer between the residential areas north of Conser Road NE and future industrial development.

The scope of work included the following:

- Review geologic data along project alignment.
- Perform a subsurface exploration by drilling six soil borings and excavating six test pits located along the project alignment. Field geotechnical data collection was performed by Jacobs.
- Perform laboratory testing on selected soil samples to evaluate engineering properties of subsurface materials along the project alignment.
- Prepare this Geotechnical Data Report.

The collected soil data will be interpreted to provide geotechnical recommendations to aid in the design and construction of the project. The geotechnical interpretation of the data and recommendations for design criteria for the facilities will be presented in a separate Geotechnical Recommendations Report by Jacobs.

2. Project Description

NE Transition Parkway will be extended approximately 0.7 mile from the existing eastern segment of NE Transition Parkway, adjacent to Millersburg Fire Station 15, to Conser Road NE, as shown on Figure 1. The storm drain system and potable water pipelines will be routed through City right-of-way along new NE Transition Parkway. The potable water pipeline will consist of 12-inch ductile iron and 20-inch high-density polyethylene water lines and connections to existing lines. The storm drain pipeline will consist of 12-inch and 36-inch ductile iron and polyvinyl chloride materials.

The pipelines will be constructed using conventional open-cut trench excavation methods for the majority of the pipeline. One trenchless installation is currently proposed beneath the BNSF Railway tracks.

3. Review of Local Geologic and Soil Data

Before the geotechnical exploration, geologic maps and soil reports were reviewed. Geologic information for the site and the surrounding area was obtained from the *Preliminary Geologic Maps of The Albany Quadrangle, Linn, Marion, and Benton Counties, Oregon* (Wiley 2006).

Surficial deposits in the vicinity of the site are mapped as Willamette silt (Q_{ws}). The Willamette silt deposits typically consist of thin- to medium-bedded rhythmites of silt, sandy silt, and silty clay that were caused by repeated Missoula (Bretz) floods when glacial dams in the upper Columbia River drainage catastrophically failed and generated floodwaters that temporarily filled the Willamette Valley. Beneath the Willamette silt older, buried, soils and weathered zones are often preserved along the top of the buried unit. Underlying the Quaternary Willamette silt is a late Pleistocene deposit consisting of Stratified bench gravel, sand, and mud, locally referred to as Leffler Gravel (Q_{le}). The thickness of Leffler gravels is estimated up to 50 feet. The Willamette silt and Leffler gravels are underlain by the Spencer Formation (T_{es}). The Spencer Formation consists of shallow marine sandstone, pebbly sandstone, conglomerate, siltstone, claystone, and coal.

Soil reports available from the National Resources Conservation Service provide information on shallow soil materials to about 6 feet deep. The mapped Willamette silt on the Natural Resources Conservation Service interactive geologic map corresponds with the Amity silt loam, Willamette Silt loam, and Woodburn silt loam soils in the soil report covering the project alignment (NRCS n.d.).

4. Field Exploration

Jacobs conducted a geotechnical exploration for Millersburg Transition Parkway in January 2023 to collect soil samples for laboratory testing and to observe soil materials and conditions. Four soil borings and six test pits were explored along the project alignment and two deep soil borings were drilled at trenchless crossing locations near the proposed launch and receiving pits as summarized in Table 1. Figure 2 shows the approximate locations of the boreholes. Soil boring logs are included in Appendix A.

Before starting the geotechnical subsurface exploration program, Jacobs notified the Oregon Utility Notification Center and obtained the necessary utility clearances. The borings were located in the field by Jacobs and adjusted to avoid utility conflicts.

Table 1. Summary of Geotechnical Exploration

Boring No.	Latitude	Longitude	Ground Surface Elevation (feet)	Completion Depth (feet bgs)	Exploration Method Type	Location
B-1-23	44.6808	-123.0762	234	21.5	Hollow Stem Auger	East of power station
B-2-23	44.6808	-123.0726	231	21.5	Hollow Stem Auger	South of Conser Road NE
B-3-23	44.6808	-123.0670	241	21.5	Hollow Stem Auger	South of Conser Road NE
B-4-23	44.6808	-123.0642	225	21.5	Hollow Stem Auger	South of Conser Road NE
B-5-23	44.6796	-123.0775	212	61.5	Sonic Core	South of power station
B-6-23	44.6793	-123.0787	211	61.5	Sonic Core	Southwest of substation on the other side of rail
TP-1-23	44.6808	-123.0753	233	10	Excavation	South of Conser Road NE
TP-2-23	44.6808	-123.0739	235	10	Excavation	South of Conser Road NE
TP-3-23	44.6808	-123.0670	229	10	Excavation	South of Conser Road NE
TP-4-23	44.6808	-123.0656	225	10	Excavation	South of Conser Road NE
TP-5-23	44.6804	-123.0644	224	10	Excavation	South of Conser Road NE
TP-6-23	44.6796	-123.0644	225	10	Excavation	Southwest of fire station

Abbreviations:

bgs = below ground surface

The exploratory test borings were advanced using a hollow-stem auger drilling and sonic core drilling method with an 8-inch-diameter auger and a truck-mounted CME-850, as well as an 8-inch-diameter barrel and a track-mounted Geoprobe 8150 drill rig. The test pits were excavated using a Deere 50G compact excavator. The borings and test pits were located as close to the proposed project alignment as possible considering equipment accessibility, utility easements, encroachment, and property access. Locations were measured from existing site features during the field investigation and have not been surveyed. The locations and elevations are approximate.

In situ testing and soil sampling were performed in the soil borings using standard penetration test split-spoon samplers at 5-foot intervals. The standard penetration test was performed in accordance with ASTM International (ASTM) D1586, Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils (ASTM 2022). The blow counts recorded on the logs and graphical profiles in this report

are the actual blows per foot recorded during drilling and are not corrected for overburden or hammer energy.

The split-spoon samplers provide a disturbed sample of the soil and an empirical indication (N-value) of the soil density or consistency. The Jacobs field representative provided continuous field observations and logging of the soils encountered. Visual classification of collected soil samples was completed in accordance with ASTM D2488, Standard Practice for Description and Identification of Soils (Visual-Manual Procedure) (ASTM 2018a). Sample descriptions, results of field tests, and observations made during drilling were recorded on the soil boring and test pit logs. Where laboratory tests were available, the soil classification was re-evaluated and updated if necessary.

5. Soil Laboratory Testing

FEI Testing & Inspection, Inc of Corvallis, Oregon, performed a series of laboratory tests on representative soil samples recovered from the soil borings and the test pit. The visual field classifications were modified, as necessary, based on laboratory test results. The following laboratory tests were conducted:

- Natural moisture content in accordance with ASTM D-2216, Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass (ASTM 2019)
- Percent finer than No. 200 sieve in accordance with ASTM D-1140, Standard Test Methods for Determining the Amount of Material Finer than 75-μm (No. 200) Sieve in Soils by Washing (ASTM 2017a)
- Particle-size analysis in accordance with ASTM D-6913, Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis (ASTM 2017b)
- Liquid limit, plasticity limit, and plasticity index of soils Atterberg limits in accordance with ASTM D-4318, Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils (ASTM 2018b)

Table 2 summarizes the laboratory test results, which also are presented on the field boring logs in Appendix A next to the corresponding sample. Appendix B provides the laboratory test results.

Table 2. Summary of the Laboratory Results

		Depth	USCS	Moisture		Sieve	Atterberg Limits		
Boring No.	Sample	(feet bgs)	Soil Type	Content (%)	Gravel (%)	Sand (%)	Fines (%)	Liquid Limit	Plasticity Limit
B-1-23	1-SS	2.5	CL	31.3	-	-	73.2	-	-
B-1-23	2-SS	5	CL	28.9	-	-	-	43	21
B-2-23	1-SS	2.5	CL	30.6	-	-	-	42	24
B-2-23	2-SS	5	CL	33.2	-	-	87.9	-	-
B-3-23	3-SS	10	ML	23	-	-	65	-	-
B-3-23	4-SS	15	MH	51.8	-	-	-	75	41
B-4-23	2-SS	5	ML	35.1	-	-	-	42	27
B-4-23	3-SS	10	ML	31	-	-	74.5	-	-
B-5-23	2-SS	5	CL	30.5	-	-	56.2	-	-
B-5-23	3-SS	7.5	CL	35.5	-	-	-	34	21
B-5-23	4-SS	10	CL	34.1	-	-	72.7	-	-
B-5-23	5-SS	12.5	CL	34.2	-	-	-	33	22
B-5-23	8-SS	20	CL	31.6	0	20	80	-	-
B-5-23	10-SS	30	SM	27	0	61	39	-	-
B-5-23	13-SS	45	SM	21.7	0	76	24	-	-
B-5-23	15-SS	60	SM	21.5	-	-	22	-	-
B-6-23	1-SS	2.5	CL	34.1	i		-	39	25
B-6-23	2-SS	5	CL	26.6	-	-	57.7	-	-
B-6-23	3-SS	7.5	CL	33.7	-	-	-	34	21
B-6-23	6-SS	15	СН	34.7	-	-	-	53	23
B-6-23	9-SS	25	SP-SM	13.2	41	48	11	-	-
B-6-23	11-SS	35	SP-SM	27.8	0	84	16	-	-
B-6-23	13-SS	45	SM	25.1	0	71	29	-	-
B-6-23	15-SS	60	SM	21.4	0	71	29	-	-

Abbreviations:

-- = not applicable

bgs = below ground surface

CH = fat clay

CL = lean clay

MH =elastic silt

ML = sandy silt or silt with sand

SM = silty sand

SP-SM = poorly graded sand with silt USCS = Unified Soil Classification System

General Soil Profiles 6.

The pipeline will generally be installed using conventional open-cut excavation methods with a pipe cover depth of approximately 3 to 5 feet bgs. Trenchless crossings are proposed to be installed using auger boring methods at depths of 10 to 20 feet bgs. The soil profile at the trenchless crossing is described in

Section 6.1, and is based on two borings drilled at the railroad crossing—one on the west side and one on the east side of the existing features. Detailed descriptions of soil materials encountered in the explorations are presented on the soil boring and test pit logs in Appendix A.

6.1 Railroad Crossing

The upper 22 to 25 feet bgs at the railroad crossing was Willamette silt, which generally consisted of lean clay, with a bottom layer of sandy clay with gravel sandy lean clay. The clay soil materials had a plasticity index of 11 to 14, indicating medium plasticity. The clay soils were firm to stiff, with uncorrected N-values varying from 4 to 12.

The upper clayey clay soils were underlain by Leffler gravels. Leffler gravels generally consist of poorly graded sand, silt sand, and sand with silt with varying amount of gravels present. The Leffler gravels were dense to very dense, with uncorrected N-values varying from 32 to greater than 50. Railroad crossing borings (B-5-23 and B-6-23) extended to 61.5 feet bgs and terminated in this unit.

6.2 Fire Station to Power Station Alignment

The upper 20 feet bgs along the roadway alignment also was Willamette silt, which consisted of silt, lean clay and elastic silt with trace sand. The Willamette silt soils along the alignment had a plastic index of 15 to 41, indicating medium to high plasticity and uncorrected N-values varying from 7 to 27 indicating firm to very stiff conditions. The borings and test pits along the alignment terminated in this unit.

7. Groundwater

Groundwater was not observed in borings or test pits during the January 2023 exploration program.

8. Limitations

This report has been prepared for the exclusive use of the City for specific application to the Millersburg Transition Parkway Project in accordance with generally accepted geotechnical engineering practice. No other warranty, express or implied, is made.

The data contained in this report were collected during the geotechnical subsurface investigations for the proposed roadway and pipeline. Soil borings indicate subsurface conditions only at specific locations and times, and only to the depths penetrated. They do not necessarily reflect strata variations that may exist between such locations.

Jacobs is not responsible for claims, damages, or liability associated with the subsurface data or reuse of the subsurface data without the express written authorization of Jacobs.

9. References

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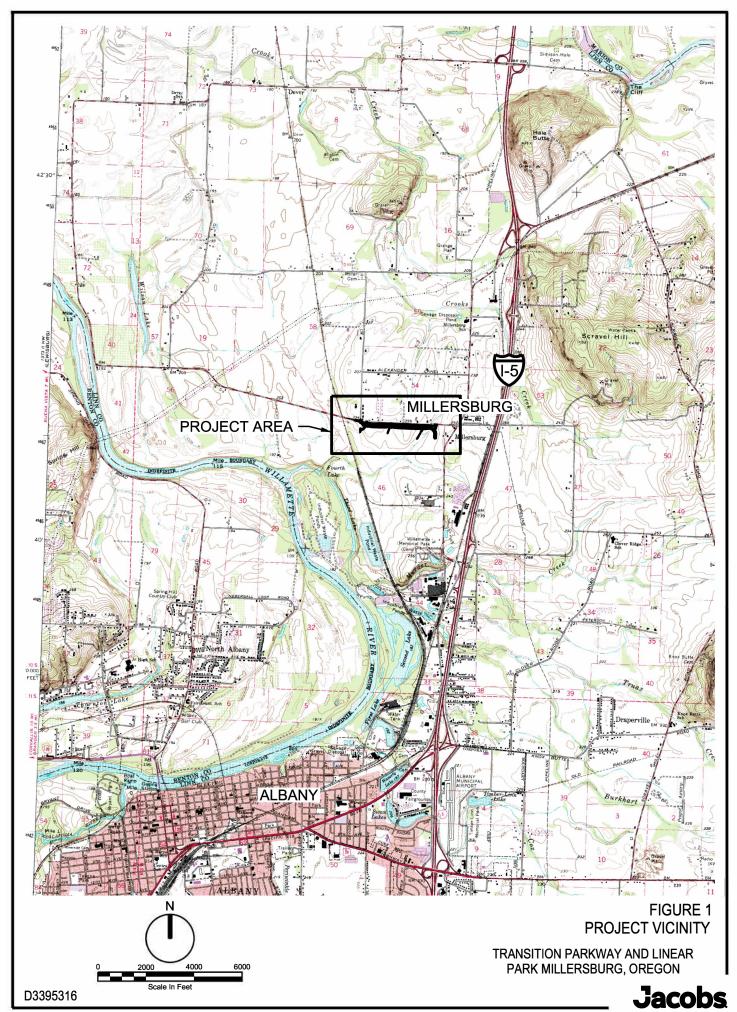
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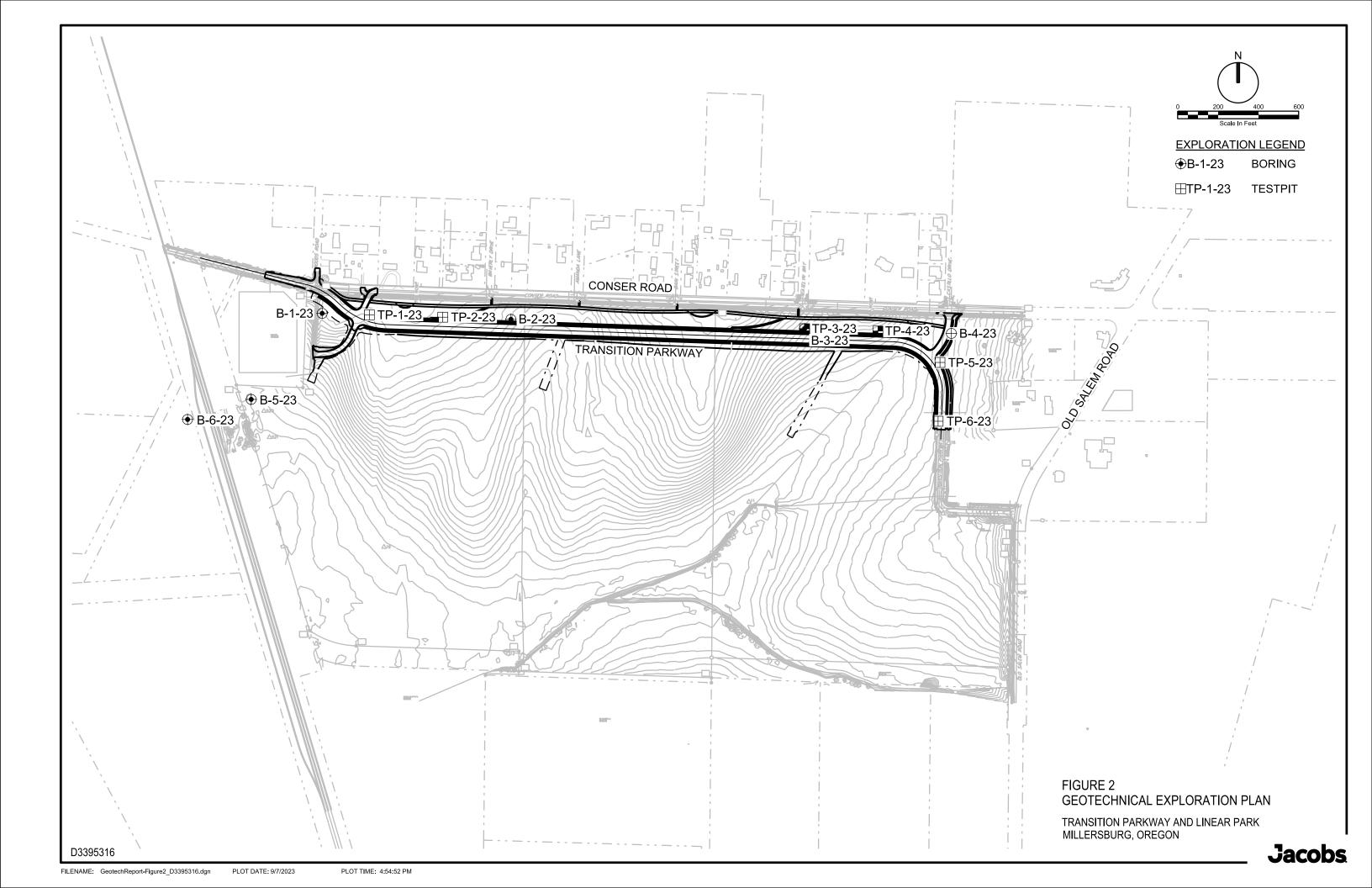
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Figures





Appendix A Ground Exploration Logs



PROJECT NUMBER:

D3395316

BORING NUMBER:

B-1-23

SHEET 1 OF 1

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 234.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD: CME-850, Track-Mounted Drill, Hollow Stem Auger ORIENTATION: Vertical

-			ND IVIL II	IOD : CIVIL-030, 1	rack-Mounted Drill, Hollow Stem Auger		ORIENTATION: Vertical
WATER	LEVELS	:		П		: 1/5/23 1	
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
234.0_					TOPSOIL/DUFF	74 12	_
- - - - -	2.5	1.5	1-SS	3-4-4 (8)	LEAN CLAY (CL) brown, moist, firm, low to medium plasticity, trace fin sand	e _	Lab Results: MC : 31.3% P200 : 73.2%
5	5.0					_	-
229.0_	6.5	1.2	2-SS	3-3-6 (9)	LEAN CLAY (CL) similar to above, except stiff	-	Lab Results: MC : 28.9% LL : 43 PI : 22
- - - 10	10.0					- - - - -	- -
224.0_	11.5	1.2	3-SS	3-5-6 (11)	LEAN CLAY (CL) similar to above		-
- - - - - 15	15.0					- - - - - -	
219.0_	16.5	1.4	4-SS	20-24-42 (66)	SANDY SILT (ML) brown, moist, hard, low to medium plasticity, fine to medium sand		- - -
20	20.0				DOORLY ODARED CAND (OR)	- - - - - - - -	- -
214.0_	21.5	1.5	5-SS	27-26-31 (57)	POORLY GRADED SAND (SP) brown gray, moist, very dense, fine to medium sand		-
-					Bottom of Boring at 21.5 ft bgs on 1/5/23 11:30	-	
25_ 209.0_ - - -							
30						-	-



PROJECT NUMBER:

D3395316

BORING NUMBER:

B-2-23

SHEET 1 OF 1

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 231.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD : CME-850, Track-Mounted Drill, Hollow Stem Auger ORIENTATION : Vertical

			ND IVIE I F	10D . CIVIE-030, 1	rack-Mounted Drill, Hollow Stem Auger			ORIENTATION : Vertical
WATER	LEVELS :				START: 1/5/23 09:40 END:	1/5/23	3 10	
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY		SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
231.0					TOPSOIL/DUFF		1 1/.	_
- - -	2.5				LEAN CLAY (CL)	-	777	Lab Desulter
-	4.0	1.5	1-SS	3-3-4 (7)	brown, moist, firm, low to medium plasticity, trace fine sand			Lab Results: MC : 30.6% LL : 42 PI : 18
5_	5.0							
226.0_ - -	6.5	1.5	2-SS	1-3-4 (7)	LEAN CLAY (CL) similar to above			Lab Results: MC : 33.2% P200 : 87.9%
10	10.0					-		- - - - - -
221.0	11.5	1.5	3-SS	2-5-6 (11)	LEAN CLAY (CL) similar to above, except stiff, ferrous staining			
- - - - -	11.5							- - - - - -
15 216.0_	15.0	1.4	4-SS	7-12-15 (27)	SANDY SILT (ML) brown, moist, very stiff, low to medium plasticity, fine	1		
20	20.0			(=- /)	to medium sand, ferrous staining			- - - - - - - -
211.0_	24.5	1.5	5-SS	16-20-36 (56)	SANDY SILT with GRAVEL (ML) brown, moist, hard, low to medium plasticity, fine to	=		 - -
25_ 206.0_	21.5			(50)	medium sand, fine gravel, ferrous staining Bottom of Boring at 21.5 ft bgs on 1/5/23 10:30			- - - - - - - - - - - - - - - - - - -
30						\dashv		-
						7		



PROJECT NUMBER: BORING NUMBER: D3395316 B-3-23 SHEET 1 OF 1

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION: (44.7 N, -123.1 E)

DRILLING CONTRACTOR: Western States Soil Conservation ELEVATION: 241.0 ft

DRILLIN	G EQUIP	MENT A	ORIENTATION : Vertical				
WATER	LEVELS	:			START: 1/5/23 08:30 END: 1/5/	23 09	2:30 LOGGER : J.S. Nair
DEPTH BELOW EXISTING GRADE (ft)		e.		STANDARD	SOIL DESCRIPTION	υ	COMMENTS
ELOV GR/	- (ft)	SAMPLE RECOVERY (ft)		PENETRATION TEST RESULTS		SYMBOLIC LOG	
H BE	₹VAL	NE.			SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR	OLIC	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND
EPT (f)	INTERVAL (ft)	SAME	#TYPE	6"-6"-6"	CONSISTENCY, SOIL STRUCTURE, MINERALOGY	YMB	INSTRUMENTATION
241.0	=	O) IL	#	(N)	TOPSOIL/DUFF	2/ 1 ^N	
					TO COLLEGE !	一]
-					-	ł	-
	2.5					<u> </u>]
-		0.4	1-SS	2-2-3	SILT (ML) brown, moist, firm, low to medium plasticity, trace fine	$\ \ $	-
	4.0	0.1	1 00	(5)	sand	Ш	_
5 -	5.0				-	ł	-
236.0	0.0			3-4-5	SILT (ML)	Ш]
-	6.5	1.5	2-SS	(9)	similar to above, except stiff	$\ \ $	-
	0.0					Г]
-					-	-	-
-						1	
-					-	ł	-
10	10.0					<u> </u>	ļ., <u>, </u>
231.0_		1.5	3-SS	5-7-10	SILT (ML) similar to above, except very stiff	$\ \ $	Lab Results: MC : 23.0%
	11.5		0 00	(17)		Ш	P200 : 65.0%
-					-	-	-
						1	
-					-	ł	-
						1]
15_ 226.0	15.0				ELASTIC SILT (MH)	Ш	Lab Results:
_		1.4	4-SS	3-5-7 (12)	brown, moist, stiff, medium to high plasticity, trace fine	Ш	MC : 51.8%
-	16.5			, ,	sand	ш	LL : 75 PI : 41
					- -	1]
-					-	ł	-
					-	1	
20 -	20.0				-	1	-
221.0_		1 =	5-SS	4-7-10	ELASTIC SILT (MH)	Ш]
-	21.5	1.5	ე-აა	(17)	similar to above, except very stiff	Ш	-
]					Bottom of Boring at 21.5 ft bgs on 1/5/23 09:30		
-					-	1	-
					-	-]
-					-	1	-
25 <u> </u>					<u>-</u>	-]
∠10.0_					-	1	-
]]					-	1]
-					-	1	-
					-]]
-					-	1	-
					-	1]
30						\vdash	_
						1	
		•	•			•	•



PROJECT NUMBER: BORING NUMBER: D3395316 B-4-23 SHEET 1 OF 1

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION: (44.7 N, -123.1 E)

DRILLING CONTRACTOR: Western States Soil Conservation

ELEVATION: 225.0 ft DRILLING FOI IIPMENT AND METHOD: Geoprobe 8150, Track-Mounted Drill, Sonic

DRILLING	G EQUIP	MENT A	ND METH	ORIENTATION : Vertical			
WATER	LEVELS	:			START : 1/4/23 08:30 END : 1/4/	/23 1	1:00 LOGGER : J.S. Nair
DEPTH BELOW EXISTING GRADE (ft)		_		STANDARD	SOIL DESCRIPTION	၂	COMMENTS
GR/	. (ft)	SAMPLE RECOVERY (ft)		PENETRATION TEST RESULTS		SYMBOLIC LOG	
H B B	INTERVAL (ft)	VE.			SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR	OLK OLK	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND
XIST	Z E	AMF	#TYPE	6"-6"-6"	CONSISTENCY, SOIL STRUCTURE, MINERALOGY	MB	INSTRUMENTATION
225.0		oπ	*	(N)	TOPSOIL/DUFF	7/1/V	•
225.0					TOFSOILIBOTT	1	1
-						-	-
1 1	2.5					1_	
-		1.4	1-SS	3-3-3	SILT (ML) brown, moist, firm, low to medium plasticity, trace fine	$\ \ $	-
1 1	4.0	1.4	1-33	(6)	sand	Ш	
5	F 0					-	-
220.0	5.0			4-4-5	SILT (ML)	╂Ш	Lab Results:
1 7	٥.	1.3	2-SS	4-4-5 (9)	similar to above, except stiff	4111	MC : 35.1% LL : 42
-	6.5			. ,		╫	PI : 15
						1	
						1	-
1 7						1]
10_	10.0					┨	-
215.0				6-7-9	SILT (ML)	1111	Lab Results:
-	11.5	1.5	3-SS	(16)	similar to above, except very stiff	$\ \ $	MC : 31.0% P200 : 74.5%
1 1	11.0					T '''	1 - 200
-						-	-
-						-	-
15_	15.0					1	
210.0		1.4	4-SS	4-4-8	SILT (ML) similar to above, except stiff	$\ \ $	-
1 1	16.5	1.4	4-33	(12)	Similar to above, except Still	Ш	
1 7						Ι	-
1 -						1	-
						1]
						1	-
20_	20.0				0117 (111)	1,	
205.0		1.5	5-SS	5-7-9	SILT (ML) similar to above, except very stiff	$\ \ $	-
1 1	21.5			(16)		1Ш	
1 -					Bottom of Boring at 21.5 ft bgs on 1/4/23 11:00	1	-
1 1						1	
-						-	-
1 1						1	
25 <u> </u>					<u> </u>	-	_
200.0					-	1	1
]						-]
1 -						1	-
1 7						1	
-						1	-
						1	
30						+	
$\overline{}$						_	



PROJECT NUMBER:

D3395316

BORING NUMBER:

B-5-23

SHEET 1 OF 3

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 212.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD : Geoprobe 8150, Track-Mounted Drill, Sonic ORIENTATION : Vertical

DRILLIN	G EQUIP	MENT A	ND METH	IOD : Geoprobe 8	150, Track-Mounted Drill, Sonic		ORIENTATION : Vertical
WATER	LEVELS	:		_	START : 1/4/23 12:00 END : 1/4	1/23 16	5:30 LOGGER : J.S. Nair
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
212.0				, ,	TOPSOIL/DUFF	7/ 1 ^N	
- - - - -	2.5	1.5	1-SS	2-5-7 (12)	LEAN CLAY (CL) brown, moist, stiff, low to medium plasticity, trace fine sand	- - - -	- - - - - -
5 207.0_ - -	5.0 6.5	1.5	2-SS	3-3-4 (7)	LEAN CLAY (CL) similar to above, except firm		Lab Results: MC : 30.5% P200 : 56.2%
- - - -	9.0	1.5	3-SS	5-2-3 (5)	LEAN CLAY (CL) similar to above	-///	Lab Results: MC : 35.5% LL : 34 PI : 13
10 202.0_ - -	10.0	1.5	4-SS	2-2-5 (7)	LEAN CLAY (CL) similar to above		Lab Results: MC : 34.1% P200 : 72.7%
- - -	12.5 14.0	1.5	5-SS	3-4-3 (7)	LEAN CLAY (CL) similar to above	- - - - - - - - - - - - - - - - - - -	Lab Results: MC : 34.2% LL : 33 PI : 11
15_ 197.0_ -	15.0 16.5	0.0	6-SS	3-3-3 (6)	No Recovery	<u> </u> - -	
- - - -	17.5 19.5		7-ST		LEAN CLAY (CL) gray, moist, medium plasticity, trace fine sand	- - - - -	- - - - -
20 192.0_ - -	20.0	1.5	8-SS	18-22-20 (42)	LEAN CLAY WITH SAND (CL) gray, moist, hard, low to medium plasticity, fine to medium sand	- (//	Lab Results: MC : 31.6% GRAVEL : 0% SAND : 20%
25	25.0					- - - - -	FINES : 80%
187.0_ - -	26.5	1.5	9-SS	16-27-31 (58)	POORLY GRADED SAND (SP) gray, moist, very dense, fine to medium sand	- - - : : :	- - - - -
-						- - - -	- - -
30_						+	



PROJECT NUMBER:

D3395316

B-5-23

SHEET 2 OF 3

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 212.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD : Geoprobe 8150, Track-Mounted Drill, Sonic ORIENTATION : Vertical

			ND IVIL II	IOD . Geoprobe o	150, Track-Mounted Drill, Sonic			ORIENTATION : Vertical
WATER	LEVELS	:		<u> </u>		: 1/4/23	16:3	
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY		SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
182.0_ -	30.0	1.5	10-SS	10-15-24 (39)	SILTY SAND (SM) gray, moist, dense, fine to medium sand	-		Lab Results: MC : 27.0%
35_	31.5			,	DOORLY ORANG WHA ORANG LOOP			GRAVEL : 0% SAND : 61% FINES : 39%
177.0_ - - - - - - 40_	36.5 40.0	1.3	11-SS	16-41-44 (85)	POORLY GRADED SAND with GRAVEL (SP) gray, moist, very dense, fine to medium sand			
172.0_ - - - - -	41.5	1.3	12-SS	16-14-22 (36)	POORLY GRADED SAND with GRAVEL (SP) similar to above, except dense			
45_ 167.0_ - - - -	45.0 46.5	1.3	13-SS	44-36-49 (85)	SILTY SAND (SM) gray, moist, very dense, fine to medium sand	- - - - - - - -		Lab Results: MC : 21.7% GRAVEL : 0% SAND : 76% FINES : 24%
50 162.0_ - -	50.0 51.5	1.2	14-SS	13-23-31 (54)	POORLY GRADED SAND with GRAVEL (SP) gray, moist, very dense, fine to medium sand	-		<u>-</u>
55_ 157.0_ - - - - - - - - - - -								



PROJECT NUMBER:

D3395316

Boring number:

B-5-23

SHEET 3 OF 3

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 212.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD: Geoprobe 8150, Track-Mounted Drill, Sonic ORIENTATION: Vertical

			ND IVIE I F	OD . Geoprobe o	150, Track-Mounted Drill, Sonic	.=		ORIENTATION: Vertical
WATER	LEVELS	:				ND : 1/4/2	3 16	
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOG	≀	SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
152.0_	60.0	1.3	15-SS	18-31-49 (80)	SILTY SAND (SM) gray, moist, very dense, fine to medium sand	-		Lab Results: MC : 21.5%
1 +	61.5			(00)	Bottom of Boring at 61.5 ft bgs on 1/4/23 16:30		111	P200 : 22.0%
					3	- 1		
1 1								
1 =								
65 <u></u> 147.0						_		_
1 -						=		-
-						-		
70								
142.0								
						=		
1 -						=		
1 7						-		
						1		
75 <u> </u>								<u>-</u>
137.0_						-		
1 7						4		
						1		
						_		
80 <u></u> 132.0						\exists		_
85						=		
85 <u></u> 127.0_						一		
						-		
90 -						-		
			l					

Jacobs

PROJECT NUMBER: BORING NUMBER: D3395316 B-6-23 SHEET 1 OF 3

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION: (44.7 N, -123.1 E)

ELEVATION: 211.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD : Geoprobe 8150, Track-Mounted Drill, Sonic ORIENTATION : Vertical								
WATER	LEVELS	:			START : 1/5/23 12:00 END : 1/5/	:00 LOGGER : J.S. Nair		
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION	
211.0_					TOPSOIL/DUFF	7/1/	_	
- - - -	2.5			2-4-3	LEAN CLAY (CL)		Lab Results: MC : 34.1%	
-	4.0	1.5	1-SS	(7)	brown, moist, firm, low to medium plasticity, trace fine sand		MC : 34.1% LL : 39	
					-		PI : 14	
5 206.0 - -	6.5	1.5	2-SS	3-4-6 (10)	LEAN CLAY (CL) similar to above, except stiff		Lab Results: MC : 26.6% P200 : 57.6%	
	7.5				-		. .	
- - -	9.0	1.5	3-SS	0-2-2 (4)	LEAN CLAY (CL) similar to above, except firm		<u>Lab Results:</u> MC : 33.7%	
10	10.0				-			
201.0_	40.0		4-ST		- - -			
-	12.0 12.5						_	
- -	14.0	1.5	5-SS	3-3-4 (7)	LEAN CLAY (CL) similar to above, except stiff		-	
15_	15.0				-		1	
196.0_	16.5	1.5	6-SS	3-4-5 (9)	LEAN CLAY (CL) gray, moist, firm, medium plasticity, trace fine sand		Lab Results: MC : 34.7% LL : 53	
-	17.5				_		PI : 30	
-	19.0	0.0	7-SS	2-3-3 (6)	No Recovery		<u>-</u>	
20 -	20.0				-		-	
191.0_	21.5	1.0	8-SS	2-14-27 (41)	SANDY CLAY with GRAVEL (CL) gray, moist, firm, medium plasticity, fine to medium sand, fine gravel			
25	25.0				-			
186.0_ - - - -	26.5	1.0	9-SS	27-31-29 (60)	POORLY GRADED SAND with SILT and GRAVEL (SP-SM) gray, moist, very dense, fine to medium sand, fine		Lab Results:	
					gravel, low to medium plasticity		SAND : 48% FINES : 11%	
-					-		-	
30					_			
30_								



PROJECT NUMBER:

D3395316

B-6-23

SHEET 2 OF 3

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 211.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD: Geoprobe 8150, Track-Mounted Drill, Sonic ORIENTATION: Vertical

			ND WETT	IOD . Geoprobe o	150, Track-Mounted Drill, Sonic			ORIENTATION: Vertical
WATER	LEVELS	:			START : 1/5/23 12:00	END : 1/5/2	3 15	
DEPTH BELOW EXISTING GRADE (ft)	INTERVAL (ft)	SAMPLE RECOVERY (ft)	#TYPE	STANDARD PENETRATION TEST RESULTS 6"-6"-6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COL MOISTURE CONTENT, RELATIVE DENSITY CONSISTENCY, SOIL STRUCTURE, MINERAL	/OR	SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
181.0_ -	30.0	1.2	10-SS	17-34-28 (62)	POORLY GRADED SAND with GRAVEL (SP) similar to above) _		-
35_ 176.0	31.5			. ,	SILTY SAND (SM)	- - - - - -		Lab Results:
- - - - - - - - - - - - - - - -	36.5	1.5	11-SS	10-18-50/5 (68/11")	gray, moist, very dense, fine to medium sand, medium plasticity	- - - - -		MC : 27.8% GRAVEL : 0% SAND : 84% FINES : 16%
171.0_ - - - - - - 45	41.5 45.0	1.2	12-SS	23-10-23 (33)	poorly graded sand with gravel (sP) gray, moist, dense, fine to medium sand, fine to medium plasticity]gravel,		
166.0_ - - - - - 50_	46.5	0.4	13-SS	50/5 (50/5")	SILTY SAND (SM) gray, moist, very dense, fine to medium sand, medium plasticity	- - - - -		Lab Results: MC : 25.1% GRAVEL : 0% SAND : 71% FINES : 29%
161.0_ - - - - 55_ 156.0_ - - -	51.5	0.3	14-SS	50/3 (50/3")	poorly graded Sand with Gravel (SP) gray, moist, very dense, fine to medium sand, gravel, low to medium plasticity) fine		
60								



PROJECT NUMBER:

D3395316

B-6-23

SHEET 3 OF 3

SOIL BORING LOG

PROJECT : Millersburg Transition Parkway, Millersburg, OR LOCATION : (44.7 N, -123.1 E)

ELEVATION: 211.0 ft DRILLING CONTRACTOR: Western States Soil Conservation

DRILLING EQUIPMENT AND METHOD : Geoprobe 8150, Track-Mounted Drill, Sonic ORIENTATION : Vertical

STANDARD SOIL DESCRIPTION COMMENTS FREE PRINTS SOIL DESCRIPTION SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL SET REJULTS SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL SET REJULTS SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUI, RELATIVE EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUITY EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUITY EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUITY EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUITY EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR, MOISTURE CONTINUITY EMBRITY OF SOIL NAME, LISCS GROUP SYMBOL, COLOR SYMBOL, CO	DRILLING	EQUIP	MENT A	ND METH	IOD : Geoprobe 8	150, Track-Mounted Drill, Sonic		ORIENTATION : Vertical
151.0	WATER LE	EVELS	:			START : 1/5/23 12:00 END :	1/5/23 1	5:00 LOGGER : J.S. Nair
151.0	DEPTH BELOW EXISTING GRADE ft)	NTERVAL (ft)	SAMPLE RECOVERY (ft)	TYPE	PENETRATION TEST RESULTS 6"-6"-6"	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR	SYMBOLIC LOG	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
Bottom of Boring at 61.5 ft bgs on 1/5/23 15:00 SAND: 71% FINES: 29% SAND: 71% FINES: 29% FINES: 29% SAND: 71% FINES: 29% FINES:	151.0_	60.0				gray, moist, very dense, fine to medium sand, low to	-11	Lab Results: MC: 21.4%
	70		0.2	15-SS	50/2 (50/2")	gray, moist, very dense, fine to medium sand, low to medium plasticity		MC : 21.4% GRAVEL : 0% SAND : 71%
	90						- - - -	



PROJECT NUMBER:	TEST PIT NUMBER:				
D3395316	TP-1-23	SHEET	1	OF	1

PROJECT: Millersburg Transition Parkway, Millersburg, OR	LOCATION: Millersburg, OR [Lat/Long: 44.6808, -123.0753]
ELEVATION: 233.0 ft	CONTRACTOR: Western States Soil Conservation

WATER	LEVELS: I	None	APPROX. DIMENSIONS:			LOGGER: J.S. Nair
	SAMI		SOIL DESCRIPTION			COMMENT
DEPTH BELOW SURFACE (FT)	INTERVAL (ft)	#TYPE	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	GRAPHIC LOG		DIFFICULTY IN EXCAVATION, RUNNING GRAVEL CONDITION, COLLAPSE OF WALLS, SAND HEAVE, DEBRIS ENCOUNTERED, WATER SEEPAGE GRADATIONAL CONTACTS, TEST. INSTRUMENTATION 11:00 started excavation
			TOPSOIL/DUFF	71.1%	<u></u>	11:00 started excavation
-	ł		SILT (ML)	1	\prod	-
_	.		brown, moist, low to medium plasticity, trace fine sand			-
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"-	<u> </u>		Bottom of Boring at 10.0 ft bgs on			11:30 finished excavation
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PROJECT NUMBER:	TEST PIT NUMBER:				
D3395316	TP-2-23	SHEET	1	OF	1

PROJECT: Millersburg Transition Parkway, Millersburg, OR	LOCATION: Millersburg, OR [Lat/Long: 44.6808, -123.0739]
ELEVATION: 235.0 ft	CONTRACTOR: Western States Soil Conservation

EXCAVA	TION EQU	JIPIVIEN I :	DEERE 50G DATE EXCAVATED: 1/6/2023		
WATER	LEVELS: I		APPROX. DIMENSIONS:		LOGGER: J.S. Nair
	SAMI	PLE	SOIL DESCRIPTION		COMMENT
DEPTH BELOW SURFACE (FT)	INTERVAL (ft)	#TYPE	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	GRAPHIC LOG	DIFFICULTY IN EXCAVATION, RUNNING GRAVEL CONDITION, COLLAPSE OF WALLS, SAND HEAVE, DEBRIS ENCOUNTERED, WATER SEEPAGE GRADATIONAL CONTACTS, TEST. INSTRUMENTATION
			TOPSOIL/DUFF	7, 1 ^N 7, 1	10:30 started excavation
-			SILT (ML) brown, moist, low to medium plasticity, trace fine sand		-
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-			Bottom of Boring at 10.0 ft bgs on		11:00 finished excavation
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15					



PROJECT NUMBER:	TEST PIT NUMBER:					
D3395316	TP-3-23	SHEET	1	OF	1	

PROJECT: Millersburg Transition Parkway, Millersburg, OR	LOCATION: Millersburg, OR [Lat/Long: 44.6808, -123.0670]
ELEVATION: 220 0 ft	CONTRACTOR: Western States Sail Concernation

			APPROX. DIMENSIONS:				LOGGER: J.S. Nair	
WATER L	SAMF	None DIF	SOIL DESCRIPTION	Н			LOGGLIN. J.G. INAII	\neg
DEPTH BELOW SURFACE (FT)	INTERVAL (ft)	#TYPE	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY		GRAPHIC LOG		COMMENT DIFFICULTY IN EXCAVATION, RUNNING GRAVEL CONDITION, COLLAPSE OF WALLS, SAND HEAVE, DEBRIS ENCOUNTERED, WATER SEEPAGE GRADATIONAL CONTACTS, TEST. INSTRUMENTATION 10:00 started excavation	
			TOPSOIL/DUFF	71.12	χ <u>/</u>		10:00 started excavation	
-			SILT (ML)	/ ₍₁₁	111	$\prod_{i=1}^{N}$		-
			brown, moist, low to medium plasticity, trace fine sand	Ш				
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] 3-				Ш				-1
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				Ш				
			SILT with GRAVEL (ML)	Π	Ш	П		-
-			brown, moist, low to medium plasticity, fine to coarse gravel, trace fine sand					-
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			Dathara of Davis and 40.0 ft have an	Г			10:30 finished excavation	
-			Bottom of Boring at 10.0 ft bgs on	l				-
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PROJECT NUMBER:	TEST PIT NUMBER:				
D3395316	TP-4-23	SHEET	1	OF	1

PROJECT: Millersburg Transition Parkway, Millersburg, OR	LOCATION: Millersburg, OR [Lat/Long: 44.6808, -123.0656]
ELEVATION: 225.0 ft	CONTRACTOR: Western States Soil Conservation

WATER I	WATER LEVELS: None		APPROX. DIMENSIONS:	LOGGER: J.S. Nair			
	SAMI	PLE	SOIL DESCRIPTION			COMMENT	
DEPTH BELOW SURFACE (FT)	INTERVAL (ft)	#TYPE	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	GRAPHIC LOG		DIFFICULTY IN EXCAVATION, RUNNING GRAVEL CONDITION, COLLAPSE OF WALLS, SAND HEAVE, DEBRIS ENCOUNTERED, WATER SEEPAGE GRADATIONAL CONTACTS, TEST. INSTRUMENTATION 09:30 started excavation	
			TOPSOIL/DUFF	17 77 17 71 18 71	. ' <u>/</u> .'	09:30 started excavation	
-			SILT (ML)		ΠÌ		-
-			brown, moist, low to medium plasticity, trace fine sand				-
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10				ШШ	Щ	10:00 finished excavation	_
-			Bottom of Boring at 10.0 ft bgs on				-
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PROJECT NUMBER:	TEST PIT NUMBER:				
D3395316	TP-5-23	SHEET	1	OF	1

PROJECT: Millersburg Transition Parkway, Millersburg, OR	LOCATION: Millersburg, OR [Lat/Long: 44.6804, -123.0644]
ELEVATION: 224.0 ft	CONTRACTOR: Western States Soil Conservation

EXCAVA	TION EQU	JIPIVIEN I :	DEERE 50G DATE EXCAVATED: 1/6/2023					
WATER	LEVELS: I		APPROX. DIMENSIONS:		LOGGER: J.S. Nair			
	SAMI	PLE	SOIL DESCRIPTION	1	COMMENT			
DEPTH BELOW SURFACE (FT)	INTERVAL (ft)	#TYPE	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	GRAPHIC LOG	DIFFICULTY IN EXCAVATION, RUNNING GRAVEL CONDITION, COLLAPSE OF WALLS, SAND HEAVE, DEBRIS ENCOUNTERED, WATER SEEPAGE GRADATIONAL CONTACTS, TEST. INSTRUMENTATION			
			TOPSOIL/DUFF	7118	2 09:00 started excavation			
-			SILT (ML) brown, moist, low to medium plasticity, trace fine sand		-			
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-	4.0	1-GRAB			-			
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10_								
			Bottom of Boring at 10.0 ft bgs on		09:30 finished excavation			
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PROJECT NUMBER:	TEST PIT NUMBER:				
D3395316	TP-6-23	SHEET	1	OF	1

PROJECT: Millersburg Transition Parkway, Millersburg, OR	LOCATION: Millersburg, OR [Lat/Long: 44.6796, -123.0644]
ELEVATION: 225.0 ft	CONTRACTOR: Western States Soil Conservation

EXCAVA	I ION EQU	JIPIVIEN I :	DEERE 50G DATE EXCAVATED: 1/6/2023		
WATER	LEVELS: I		APPROX. DIMENSIONS:		LOGGER: J.S. Nair
	SAMI	PLE	SOIL DESCRIPTION		COMMENT
DEPTH BELOW SURFACE (FT)	INTERVAL (ft)	#TYPE	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	GRAPHIC LOG	DIFFICULTY IN EXCAVATION, RUNNING GRAVEL CONDITION, COLLAPSE OF WALLS, SAND HEAVE, DEBRIS ENCOUNTERED, WATER SEEPAGE GRADATIONAL CONTACTS, TEST. INSTRUMENTATION
			TOPSOIL/DUFF	7118 711	08:30 started excavation
-			SILT (ML) brown, moist, low to medium plasticity, trace fine sand		- -
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	5.0	1-GRAB			
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10_					09:00 finished excavation
			Bottom of Boring at 10.0 ft bgs on		-
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Appendix B Soil Laboratory Test Results



LETTER OF TRANSMITTAL

Date: February 6, 2023	Project No.: 2226031-600						
	Re: Millersburg Transition Pkwy.						
To: Jacobs 1100 NE Circle Blvd. Suite 300 Corvallis, OR 97330							
Attn: Jithin Nair							
Enclosed are:							
□ Report□ Copy of Letter□ Other□ Drawings□ Specifications	☑ Test Results (12 Pages Total Incl. Cover)						
These are transmitted as checked below:							
☑ For your use☑ For your review☑ As requested☐ For your files	v/approval						
Remarks: Requested laboratory testing results attached. Please call if you have any questions.							
Date Sampled: unknown Location Tested: Corvallis Laboratory Sample No.: 8819							
Copy to:	Signature:						
	Dachel Day						
	Rachel Ray President						

This report and/or enclosed test data is the confidential property of the client to whom it is addressed and pertains to the specific process and/or material evaluated. As such, information contained herein shall not be reproduced in part or full and/or any part thereof be disclosed without FEI Testing & Inspection, Inc.'s written authorization.



Percent Fines & Water Content Test (ASTM D 1140)

PROJECT NAME	Millersburg Transition Pkwy.	PROJECT NUMBER	2226031-600
		FEI SAMPLE NUMBER	8819
RECORDED BY	RR	DATE	2/3/2023
CLIENT	Jacobs	CLIENT PROJECT NUMBER	D3395316

WATER CONTENT DIR or AUX*	DIR						
SAMPLE DESIGNATION	B-1-23, 1-SS	B-2-23, 2-SS	B-3-23, 3-SS	B-4-23, 3-SS	B-5-23, 2-SS	B-5-23, 4-SS	B-5-23, 15-SS
SAMPLE DEPTH	2.5'	5.0'	10.0'	10.0'	5.0'	10.0'	60.0'
Pan Number	2	31	18	30	12	55	36
Wt. of Wet Soil + Pan (g)	339.46	375.00	353.82	306.56	358.26	325.43	478.70
Wt. of Dry Soil + Pan (g)	276.75	301.58	300.69	253.11	290.78	263.40	408.18
Wt. of Water (g)	62.71	73.42	53.13	53.45	67.48	62.03	70.52
Wt. of Pan (g)	76.35	80.20	69.98	80.85	69.57	81.33	79.75
Wt. of Dry Soil (g)	200.40	221.38	230.71	172.26	221.21	182.07	328.43
Water Content (%)	31.3%	33.2%	23.0%	31.0%	30.5%	34.1%	21.5%

TEST SAMPLE DATA									
TEST METHOD A or B	B B B B		В	В	В				
Length of Time Sample Soaked (hrs	4	4	4	4	4	4	4		
Pan Number	2	31	18	30	12	55	36		
Wet Wt. + Pan (g)	339.46	375.00	353.82	306.56	358.26	325.43	478.70		
Wet Wt. (g)	263.11	294.80	283.84	225.71	288.69	244.10	398.95		
Wt. of Pan (g)	76.35	80.20	69.98	80.85	69.57	81.33	79.75		
(A) Dry Soil (g) (Total Sample)	200.40	221.38	230.71	172.26	221.21	182.07	328.43		

AFTER WASHING										
Pan Number	2	31	18	30	12	55	36			
Dry Wt. + Pan (g)	130.13	107.01	150.81	124.70	166.50	131.11	336.01			
Wt. of Pan (g)	76.35	80.20	69.98	80.85	69.57	81.33	79.75			
(B) Wt. of Dry Soil	53.78	26.81	80.83	43.85	96.93	49.78	256.26			
(C) Total Loss (g) (No. 200) (C=A-B)	146.62	194.57	149.88	128.41	124.28	132.29	72.17			
% Fines (C/A)	73.2%	87.9%	65.0%	74.5%	56.2%	72.7%	22.0%			

^{*}DIR=Dry mass was determined directly by drying the test specimen

Equipment Used

Oven ID # 6060
Scale ID # 6067
Sieve ID # LS30B

Reviewed By

Dachel Day

Percent Fines & Water Content (ASTM D 1140)

Rev. 11-05-08

^{*}AUX=Dry mass was determined using an auxiliary water content specimen



Percent Fines & Water Content Test (ASTM D 1140)

PROJECT NAMEN	Millersburg Transition Pkv	wy. PROJECT NUMBER	2226031-600	
		FEI SAMPLE NUMBER	8819	
RECORDED BY	RR	DATE	2/3/2023	
CLIENT	Jacobs	CLIENT PROJECT NUMBER	D3395316	
WATER CONTENT DIR or AUX*	DIR			
SAMPLE DESIGNATION	B-6-23, 2-SS			
SAMPLE DEPTH	5.0'			
Pan Number	61			
Wt. of Wet Soil + Pan (g)	364.79			
Wt. of Dry Soil + Pan (g)	304.80			
Wt. of Water (g)	59.99			
Wt. of Pan (g)	79.22			
Wt. of Dry Soil (g)	225.58			
Water Content (%)	26.6%			
TEST SAMPLE DATA TEST METHOD A or B Length of Time Sample Soal	B ked (hrs 4			
Pan Number	61			
Wet Wt. + Pan (g)	364.79			
Wet Wt. (g)	285.57			
Wt. of Pan (g)	79.22			
(A) Dry Soil (g) (Total Sam	ple) 225.58			
	•			
AFTER WASHING				
Pan Number	61			
Dry Wt. + Pan (g)	174.94			
Wt. of Pan (g)	79.22			
(B) Wt. of Dry Soil	95.72			
(C) Total Loss (g) (No. 200) ((C=A-B) 129.86			
% Fines (C/A)	57.6%			

^{*}DIR=Dry mass was determined directly by drying the test specimen

Equipment Used

Oven ID # 6060
Scale ID # 6067
Sieve ID # LS30B

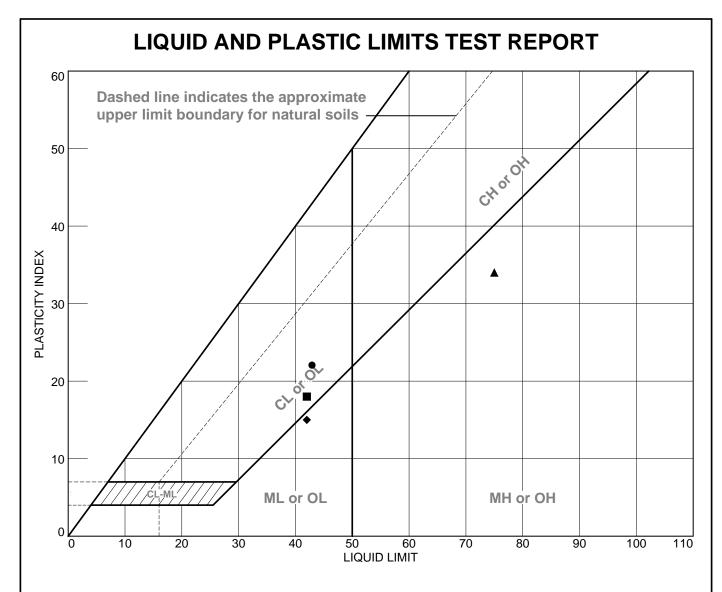
Reviewed By

Dachel Day

Percent Fines & Water Content (ASTM D 1140)

Rev. 11-05-08

^{*}AUX=Dry mass was determined using an auxiliary water content specimen



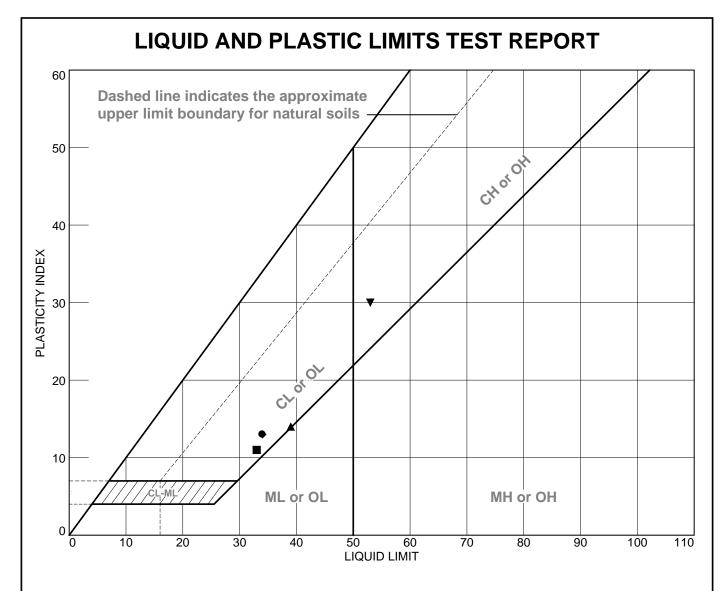
	SOIL DATA										
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	uscs			
•	8819	B-1-22, 2-SS	5.0'	28.9	21	43	22	CL			
	8819	B-2-23, 1-SS	2.5'	30.6	24	42	18	CL			
A	8819	B-3-22, 4-SS	15.0'	51.8	41	75	34	MH			
•	8819	B-4-23, 2-SS	5.0'	35.1	27	42	15	ML			

FEI Testing & Inspection, Inc.

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Corvallis, OR

Project No.: 2236031-600

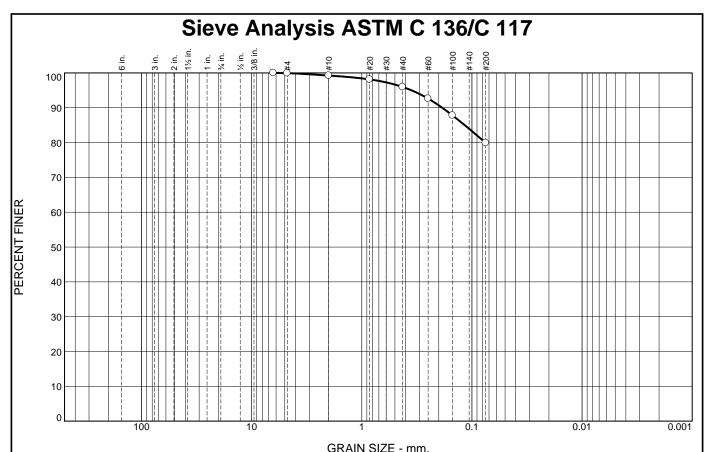


	SOIL DATA										
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS			
•	8819	B-5-23, 3-SS	7.5'	35.5	21	34	13	CL			
	8819	B-5-23, 5-SS	12.5'	34.2	22	33	11	CL			
A	8819	B-6-22, 1-SS	2.5'	34.1	25	39	14	CL			
•	8819	B-6-22, 3-SS	7.5'	33.7	21	34	13	CL			
▼	8819	B-6-22, 6-SS	15.0'	34.7	23	53	30	СН			

FEI Testing & Inspection, Inc.
Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No.: 2236031-600



9/ - 21	% G	% Gravel		% Sand		% Fines	
% +3"	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0	0	0	1	3	16	80	

PERCENT	SPEC.*	PASS?
FINER	PERCENT	(X=NO)
100		
100		
99		
98		
96		
93		
88		
80		
	FINER 100 100 99 98 96 93 88	FINER PERCENT 100 100 99 98 96 93 88

	Soil Description				
PL=	Atterberg Limits LL=	PI=			
D ₉₀ = 0.1862 D ₅₀ = D ₁₀ =	Coefficients D ₈₅ = 0.1163 D ₃₀ = C _u =	D ₆₀ = D ₁₅ = C _c =			
USCS=	Classification AASHT	O=			
Remarks Water Content: 31.6%					

(no specification provided)

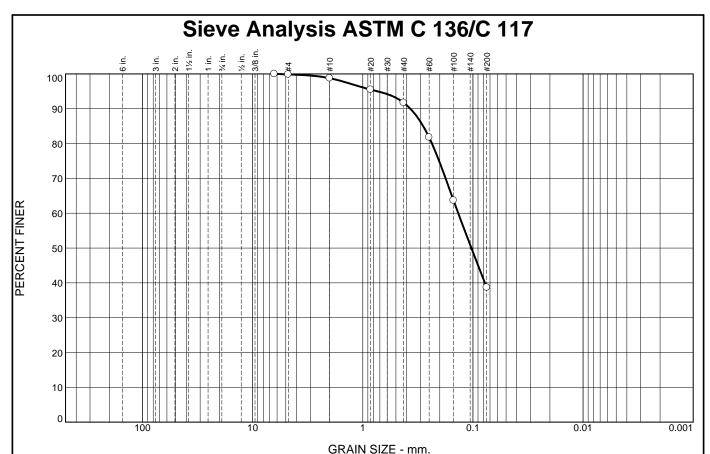
Source of Sample: 8819 Sample Number: B-5-23, 8-SS Depth: 20.0'

Sample Number: B-5-23, 8-SS Date: 02-03-2023

FEI Testing & Inspection, Inc.
Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No: 2236031-600



9/ .3"	% Gravel			% Sand		% Fines	
% +3"	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0	0	0	1	7	53	39	

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
1/4	100		
#4	100		
#10	99		
#20	95		
#40	92		
#60	82		
#100	64		
#200	39		
*			

	Soil Description				
PL=	Atterberg Limits LL=	PI=			
D ₉₀ = 0.3697 D ₅₀ = 0.1033 D ₁₀ =	Coefficients D ₈₅ = 0.2828 D ₃₀ = C _u =	D ₆₀ = 0.1360 D ₁₅ = C _c =			
USCS=	Classification AASHT	O=			
Remarks Water Content: 27.0%					

* (no specification provided)

Depth: 30.0'

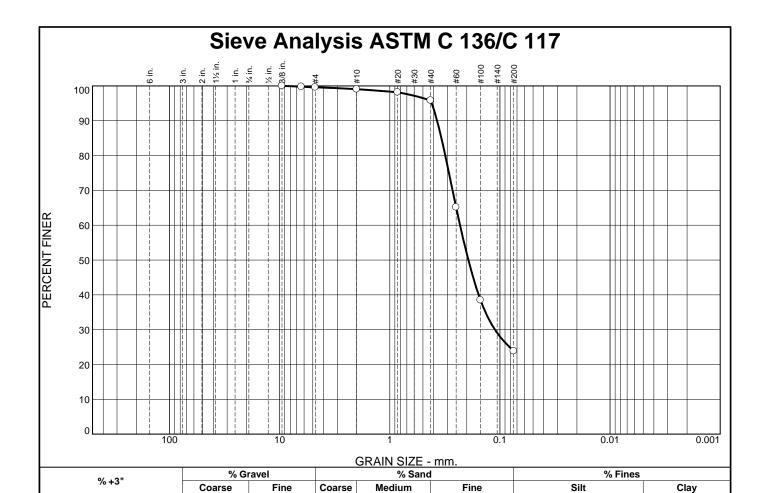
Source of Sample: 8819 Sample Number: B-5-23, 10-SS

FEI Testing & Inspection, Inc. Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No: 2236031-600

Figure



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
3/8	100		
1/4	100		
#4	100		
#10	99		
#20	98		
#40	96		
#60	65		
#100	38		
#200	24		

0

3 12	I .					
	Soil Description					
D.	Atterberg Limits	D.				
PL=	LL=	PI=				
D 0.2710	Coefficients	D 0.2202				
D ₉₀ = 0.3718 D ₅₀ = 0.1934 D ₁₀ =	D ₈₅ = 0.3398 D ₃₀ = 0.1110	D ₆₀ = 0.2303 D ₁₅ =				
D ₁₀ =	C_{u}^{30}	C _C =				
	Classification					
USCS=	AASHTO)=				
	Remarks					
Water Content: 2	Water Content: 21.7%					

* (no specification provided)

0

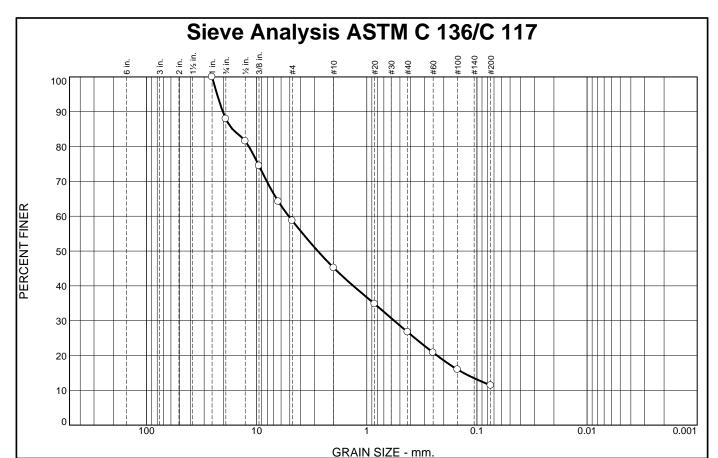
Source of Sample: 8819 Sample Number: B-5-23, 13-SS **Depth:** 45.0'

Date: 02-03-2023

FEI Testing & Inspection, Inc. Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No: 2236031-600



0/ .2"			% Gravel		% Sand		% Fines		
	% +3"	Coarse	Fin	e Coarse	Medium	Fine	Silt	Clay	
	0	12	29	14	18	16	11		
	SIEVE PE	RCENT SF	PEC.*	PASS?	Soil Description				

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
1	100		
3/4	88		
1/2	82		
3/8	74		
1/4	64		
#4	59		
#10	45		
#20	35		
#40	27		
#60	21		
#100	16		
#200	11		
	3/4 1/2 3/8 1/4 #4 #10 #20 #40 #60 #100	SIZE FINER 1 100 3/4 88 1/2 82 3/8 74 1/4 64 #4 59 #10 45 #20 35 #40 27 #60 21 #100 16	SIZE FINER PERCENT 1 100 3/4 88 1/2 82 3/8 74 1/4 64 #4 59 #10 45 #20 35 #40 27 #60 21 #100 16

	Soil Description	
PL=	Atterberg Limits LL=	Pl=
D ₉₀ = 20.2835 D ₅₀ = 2.7878 D ₁₀ =	Coefficients D ₈₅ = 16.5576 D ₃₀ = 0.5635 C _u =	D ₆₀ = 5.1086 D ₁₅ = 0.1323 C _c =
USCS=	Classification AASHTC)=
Water Content: 13	Remarks 3.2%	

(no specification provided)

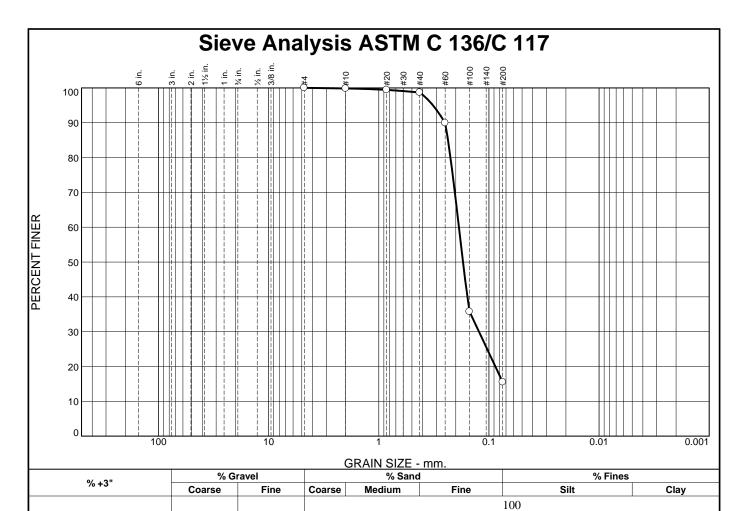
Source of Sample: 8819 Sample Number: B-6-23, 9-SS Depth: 25.0'

Date: 02-03-2023

FEI Testing & Inspection, Inc.
Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No: 2236031-600



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
#4	100		
#10	100		
#20	99		
#40	99		
#60	90		
#100	36		
#200	16		

	Soil Description		
PL=	Atterberg Limits LL=	PI=	
D ₉₀ = D ₅₀ = D ₁₀ =	Coefficients D ₈₅ = D ₃₀ = C _u =	D ₆₀ = D ₁₅ = C _c =	
USCS=	Classification AASHT	O=	
Water Content	: 27.8%		

* (no specification provided)

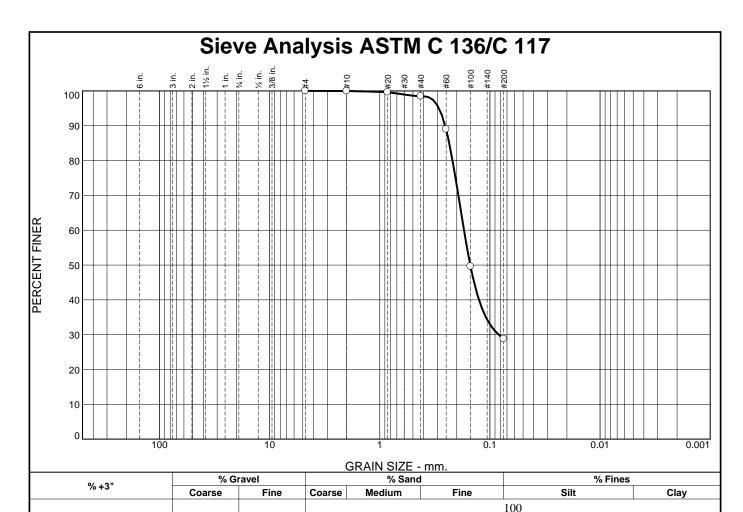
Source of Sample: 8819 Sample Number: B-6-23, 11-SS **Depth:** 35.0'

FEI Testing & Inspection, Inc. Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No: 2236031-600

Figure



ſ	SIEVE	PERCENT	SPEC.*	PASS?
	SIZE	FINER	PERCENT	(X=NO)
Ī	#4	100		
	#10	100		
	#20	100		
	#40	98		
	#60	89		
	#100	50		
	#200	29		

	Soil Description	1
PL=	Atterberg Limits	<u>s</u> Pl=
D ₉₀ = D ₅₀ = D ₁₀ =	Coefficients D ₈₅ = D ₃₀ = C _u =	D ₆₀ = D ₁₅ = C _c =
USCS=	Classification AASH	TO=
Water Content	: 25.1%	

(no specification provided)

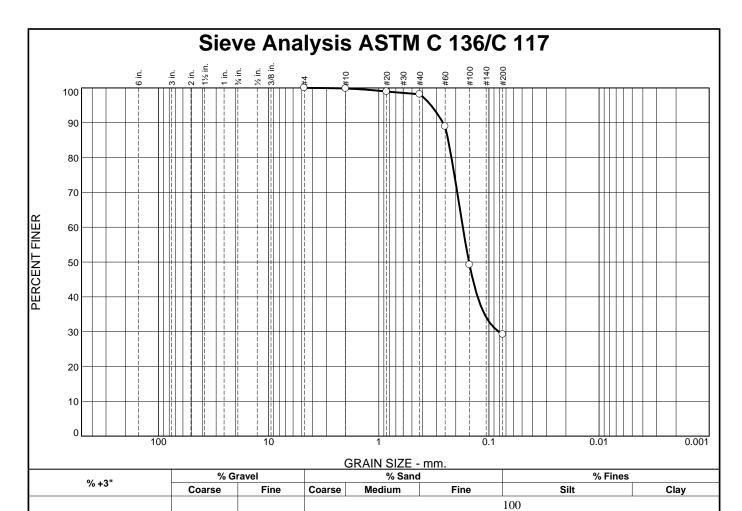
Source of Sample: 8819 Sample Number: B-6-23, 13-SS **Depth:** 45.0'

FEI Testing & Inspection, Inc. Corvallis, OR

Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

Project No: 2236031-600

Figure



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
#4	100		
#10	100		
#20	99		
#40	98		
#60	89		
#100	49		
#200	29		

	Soil Description	
PL=	Atterberg Limits LL=	PI=
D ₉₀ = D ₅₀ = D ₁₀ =	Coefficients D ₈₅ = D ₃₀ = C _u =	D ₆₀ = D ₁₅ = C _c =
USCS=	Classification AASHTO)=
Water Content	Remarks : 21.4%	

* (no specification provided)

Source of Sample: 8819 Sample Number: B-6-23, 15-SS **Depth:** 60.0'

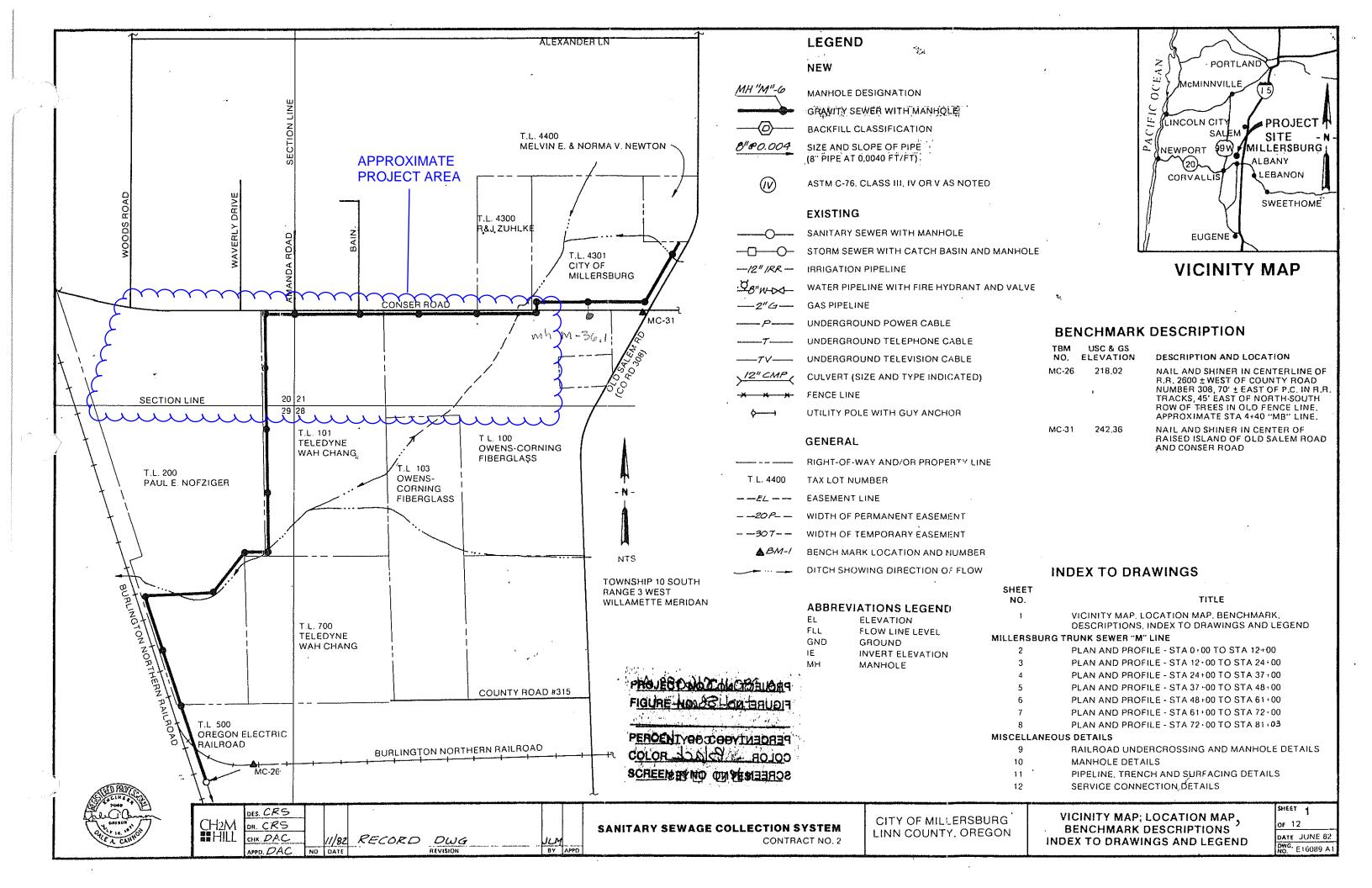
FEI Testing & Inspection, Inc. Corvallis, OR

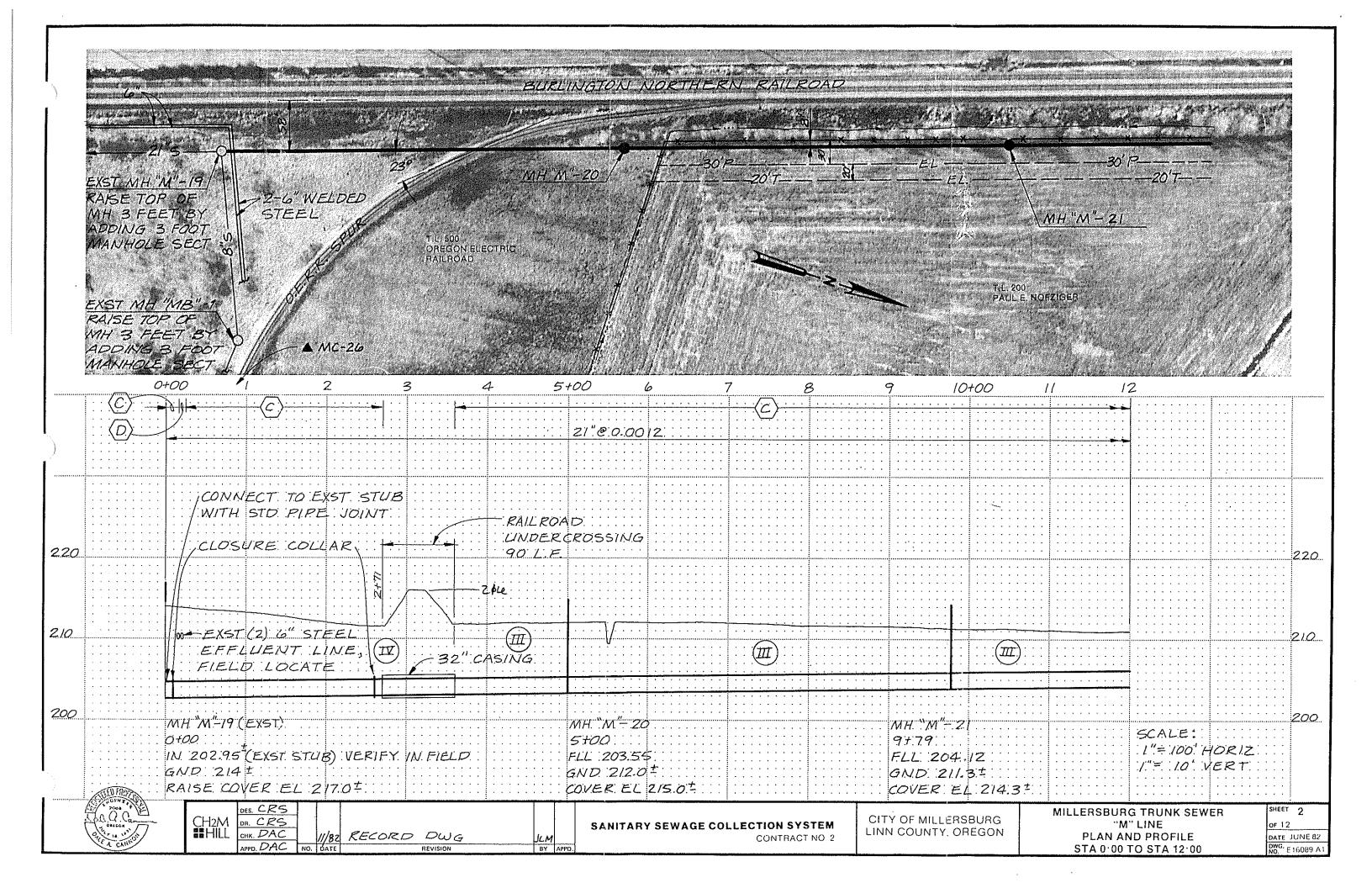
Client: Jacobs (Project No. D3395316) **Project:** Millersburg Transition Pkwy.

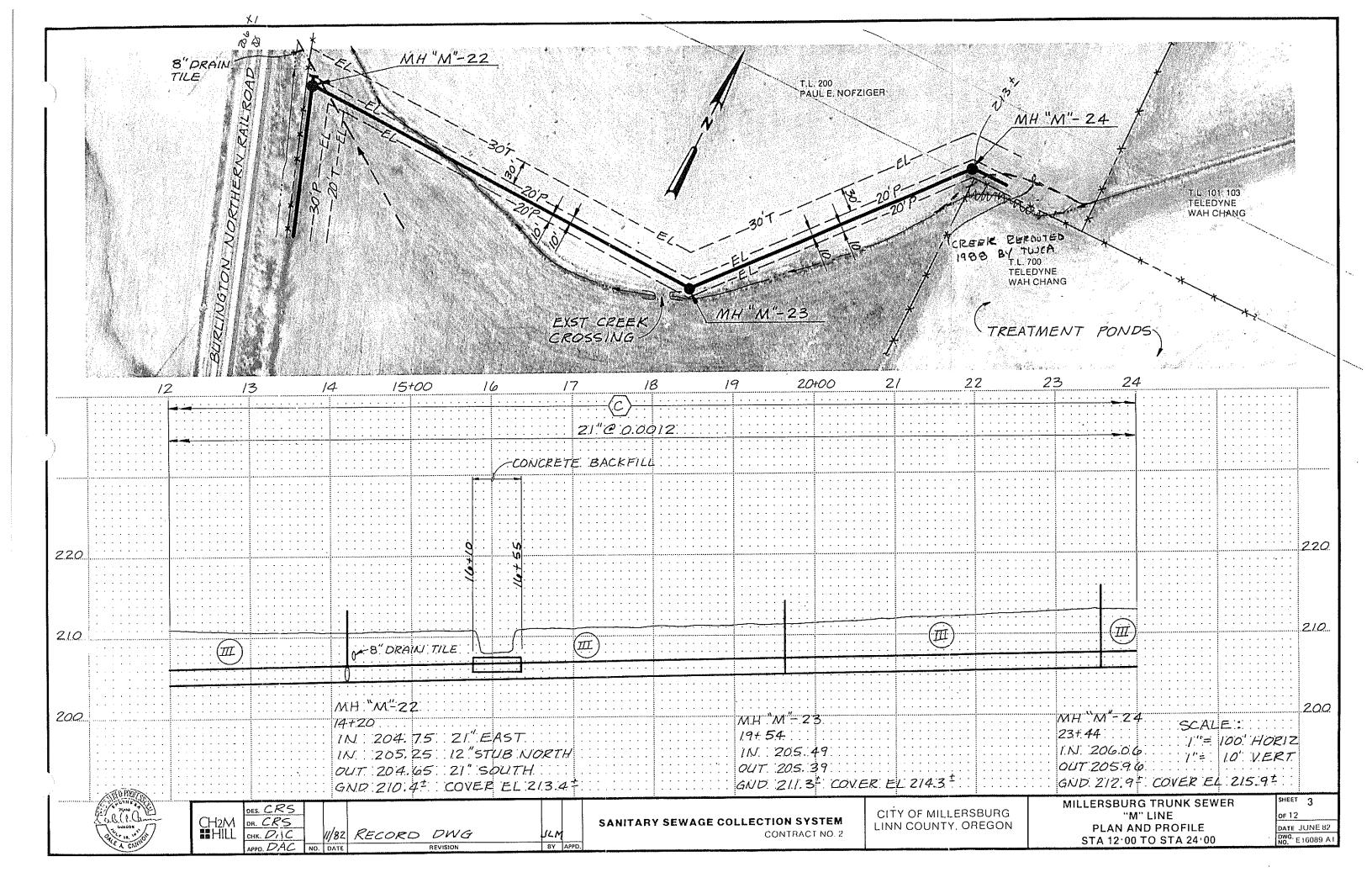
Project No: 2236031-600

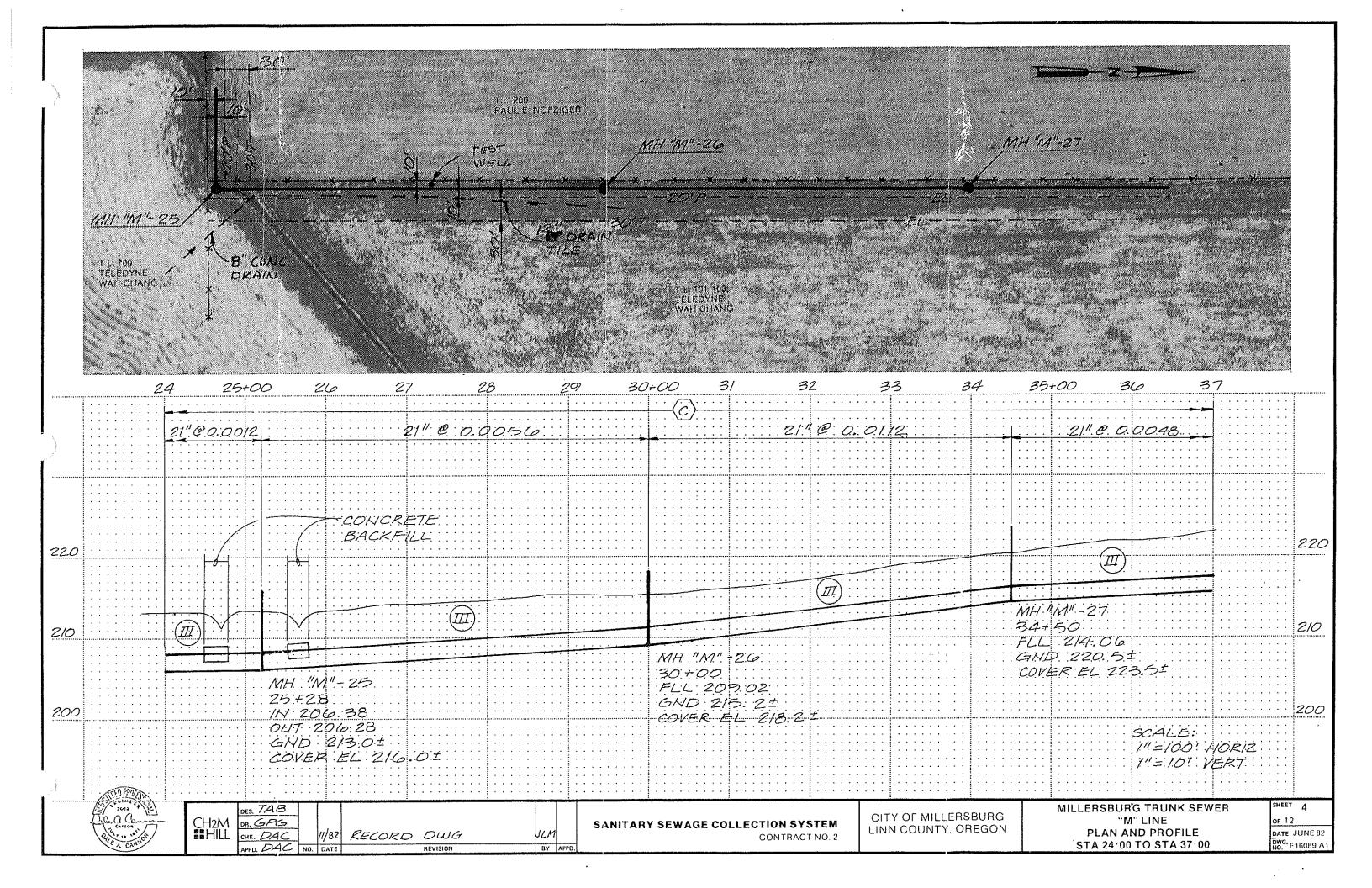
Figure

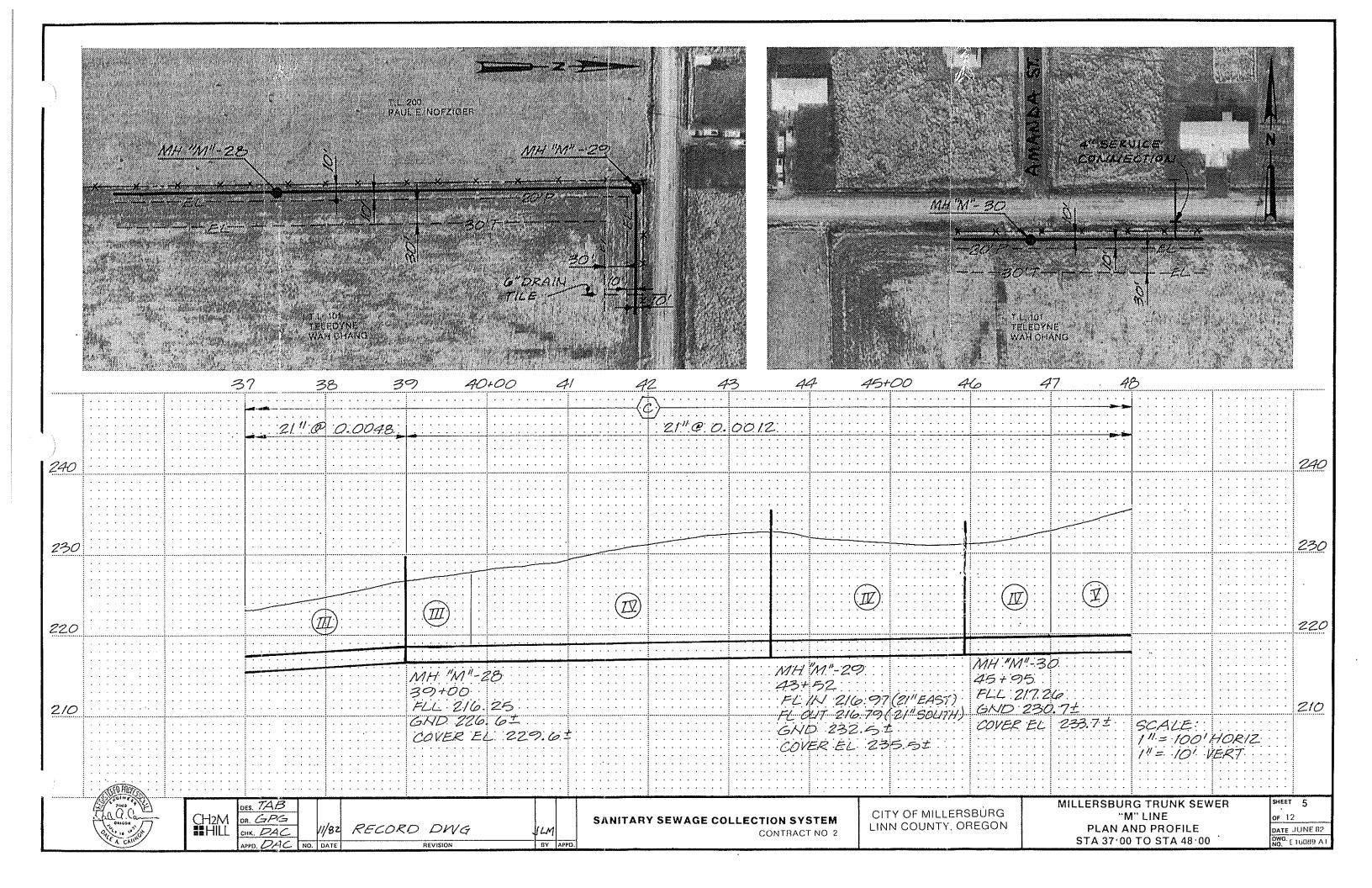
3. City of Millersburg Sanitary Sewer Collection System Contract 2

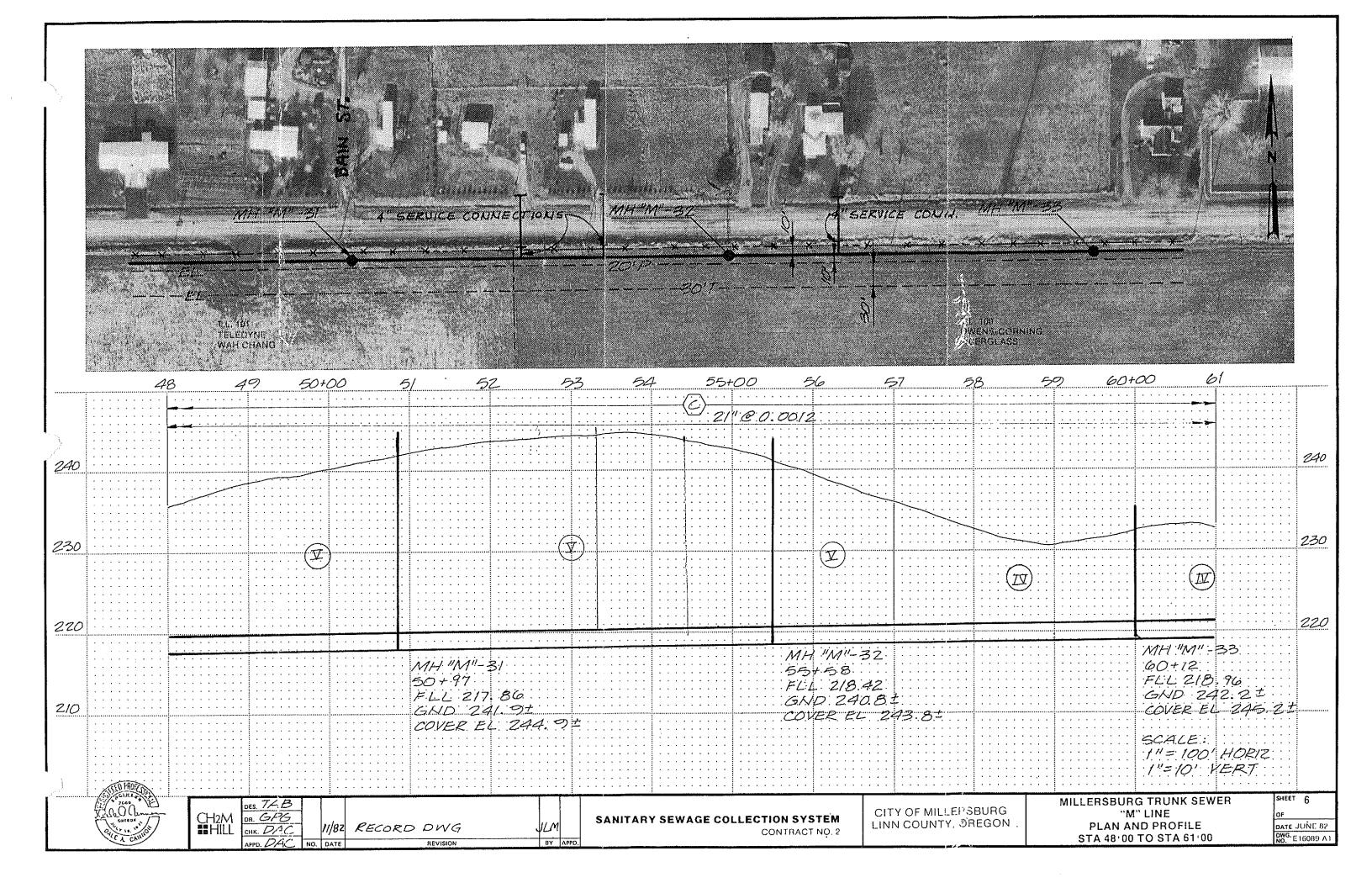


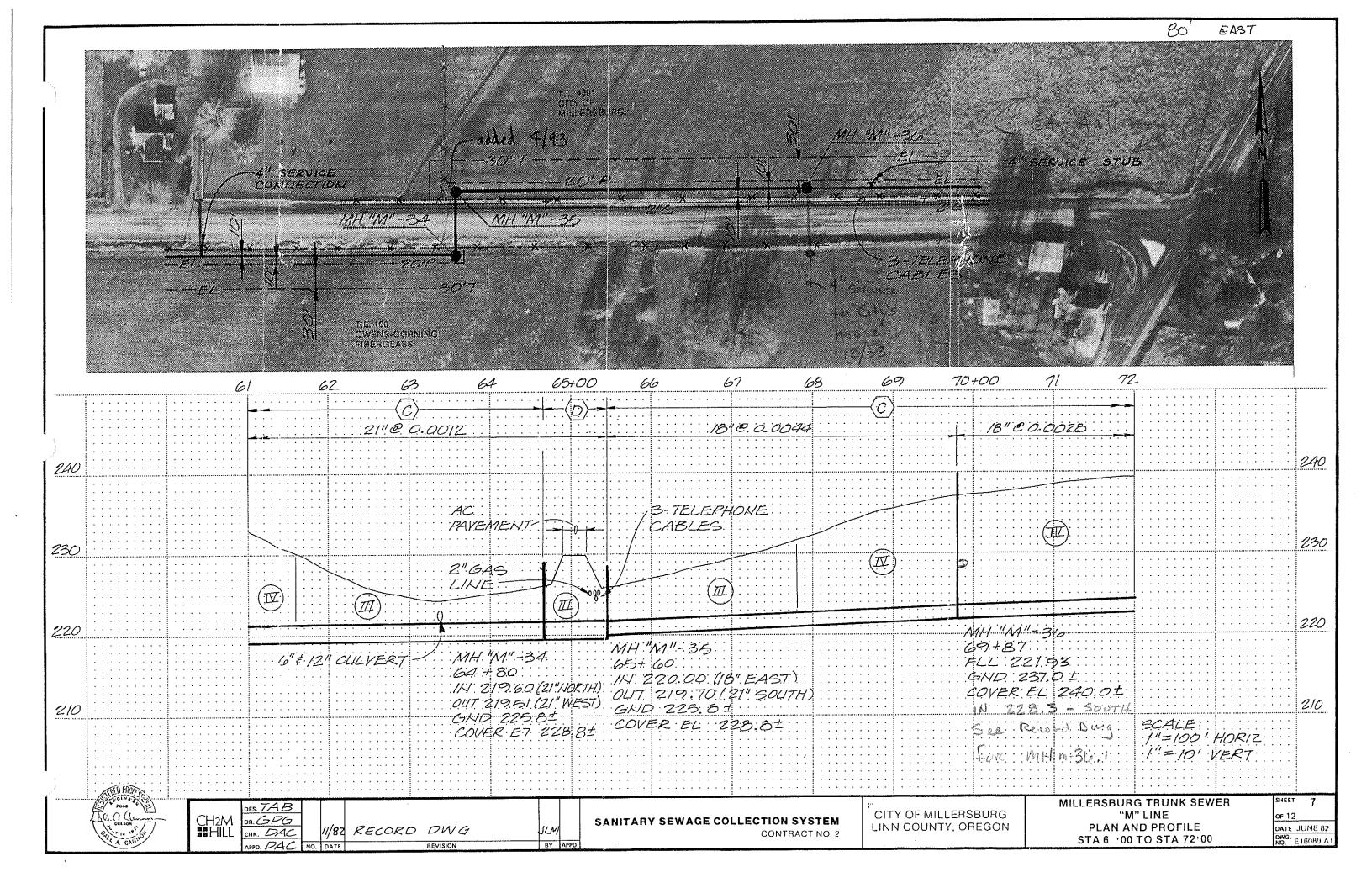


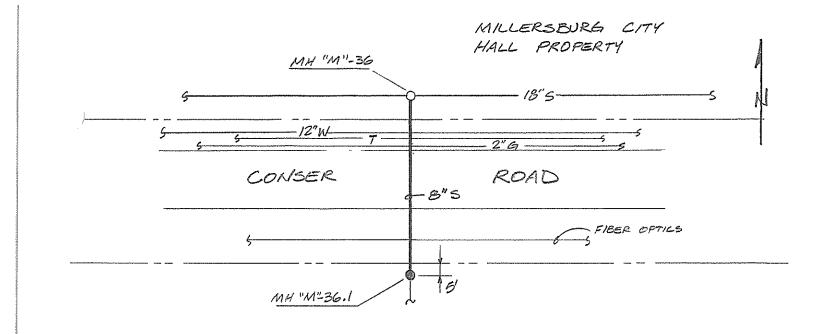


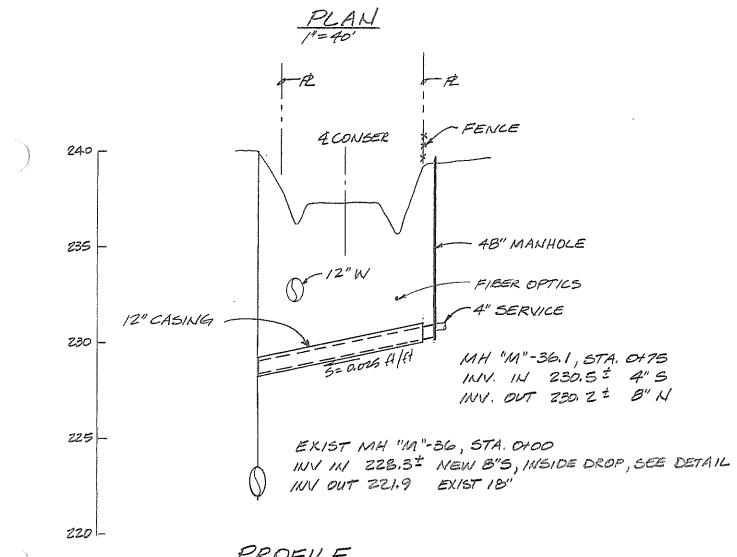










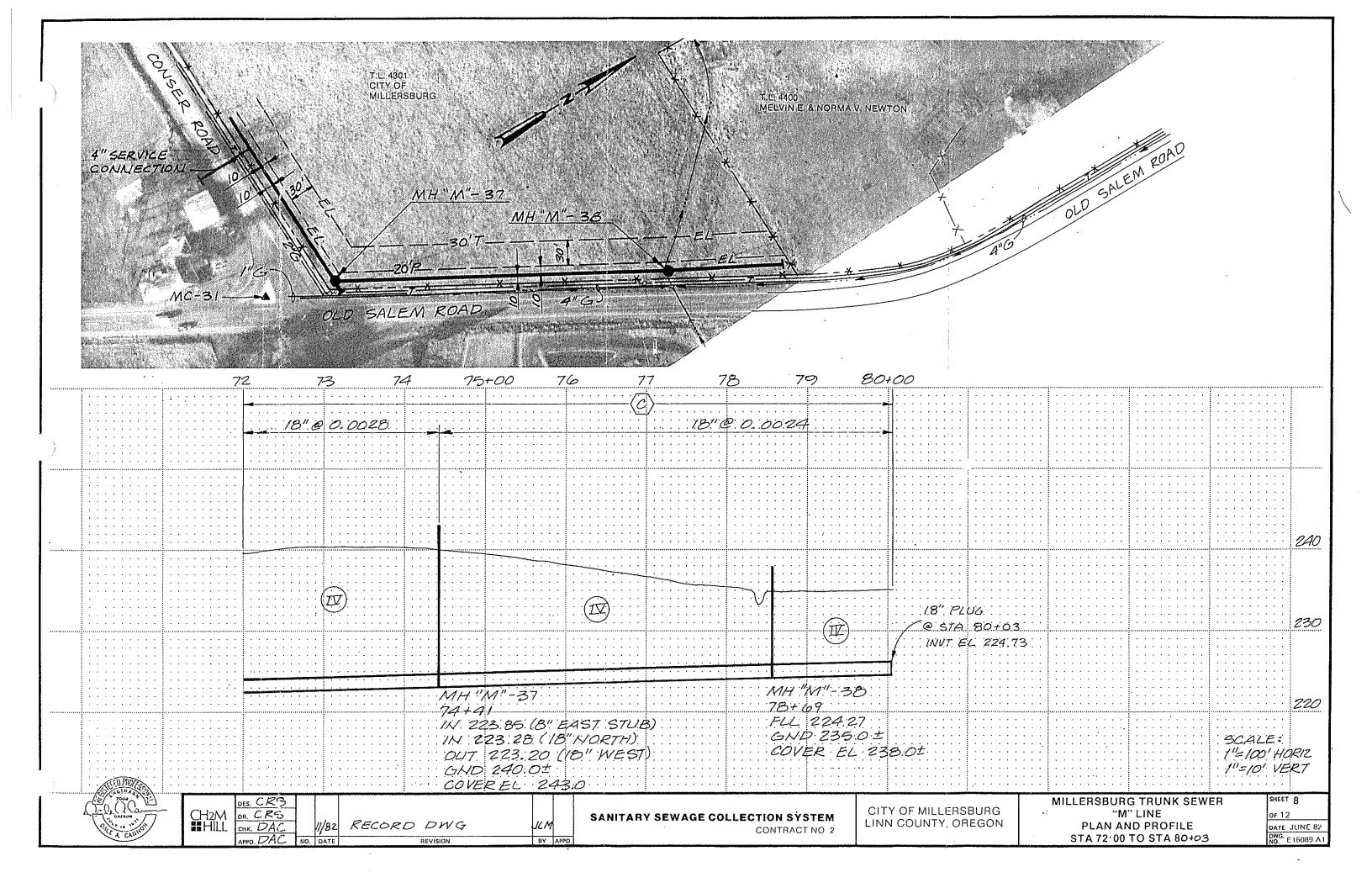


PROFILE /"=40' HORIZ /"=5 VERT

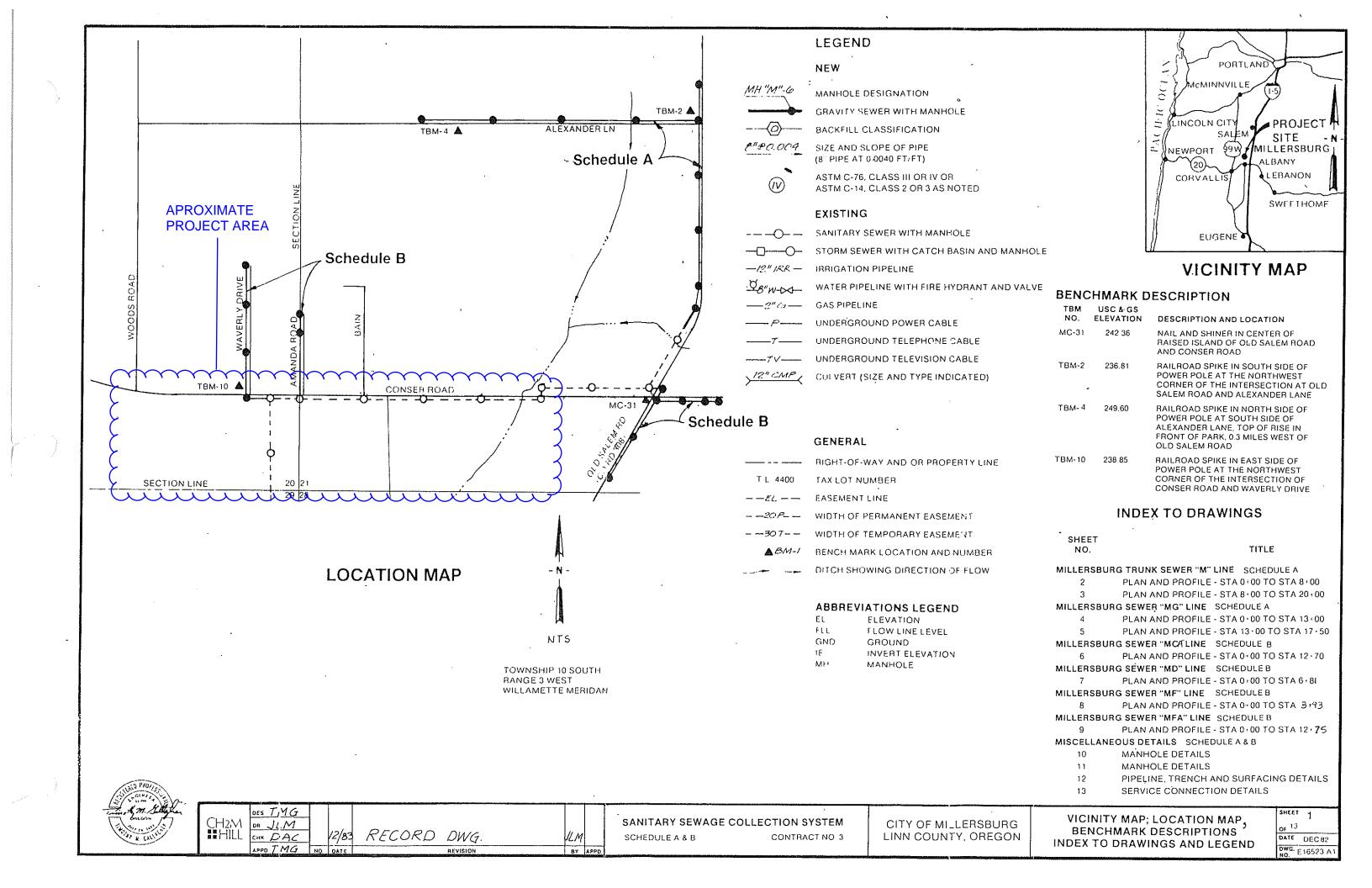
RECORD DRAWING, DEC. 2003

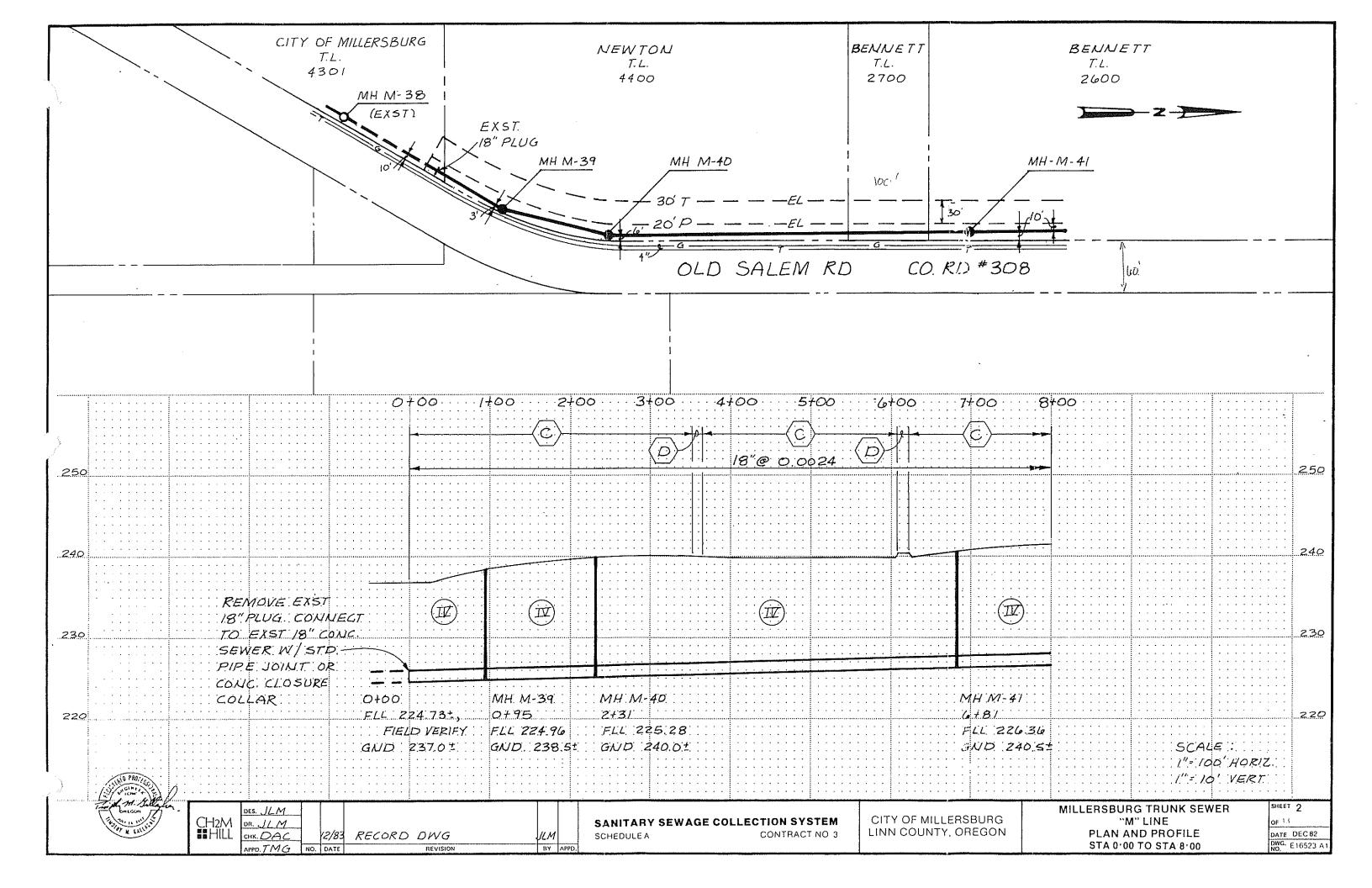
CONSER ROAD CROSSING CITY OF MILLERSBURG

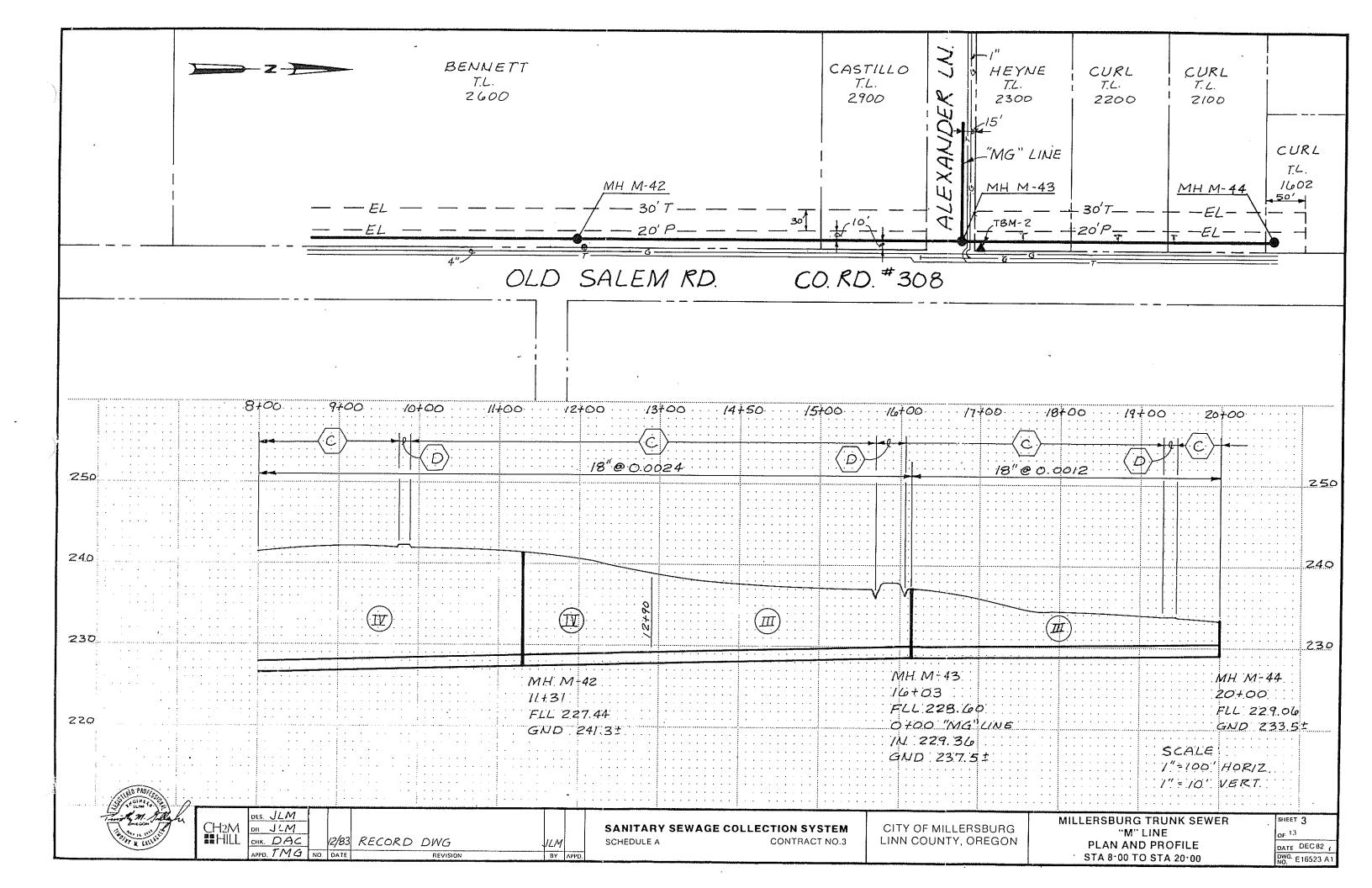
MILLERS BURG, OREGON

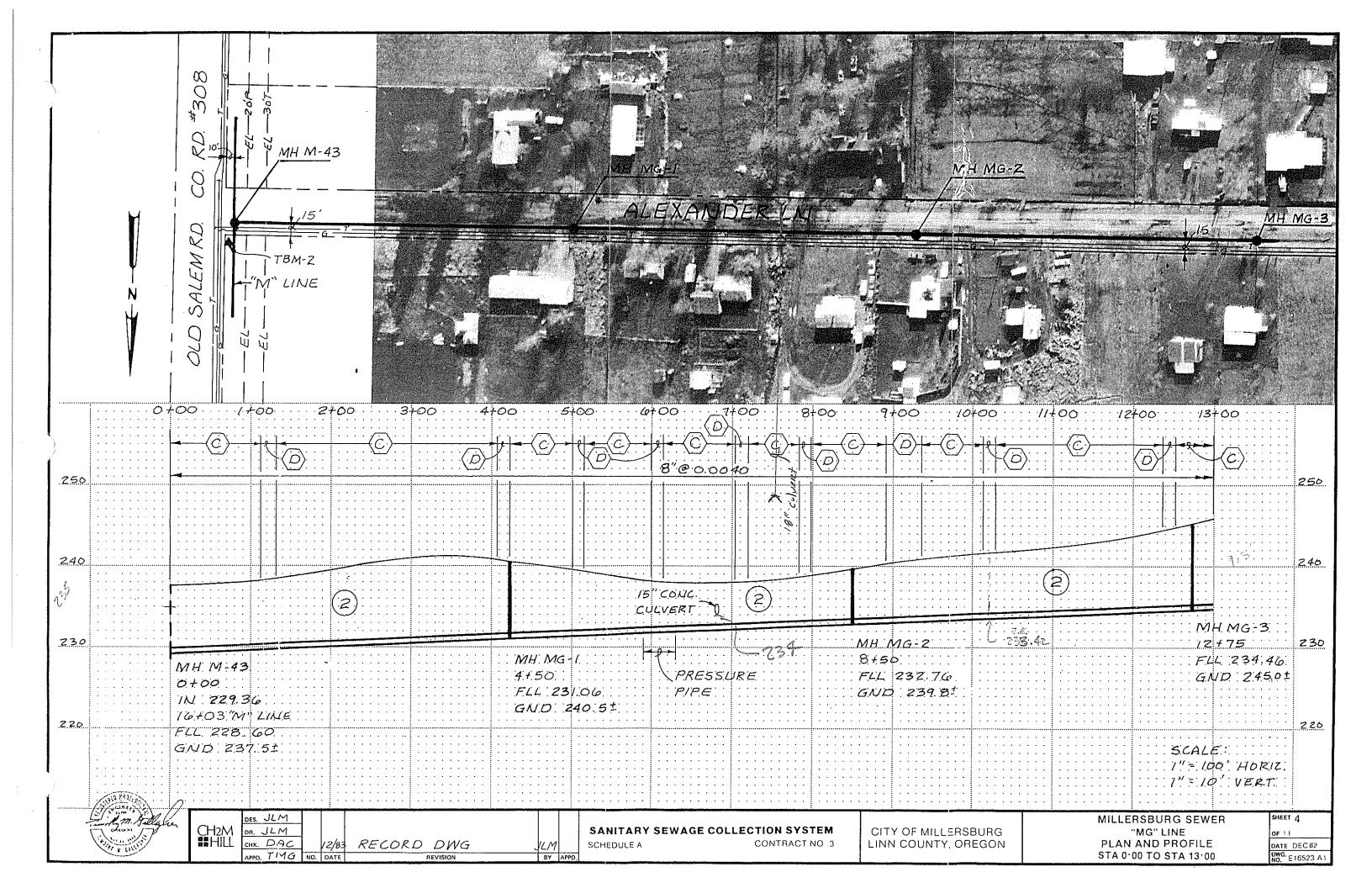


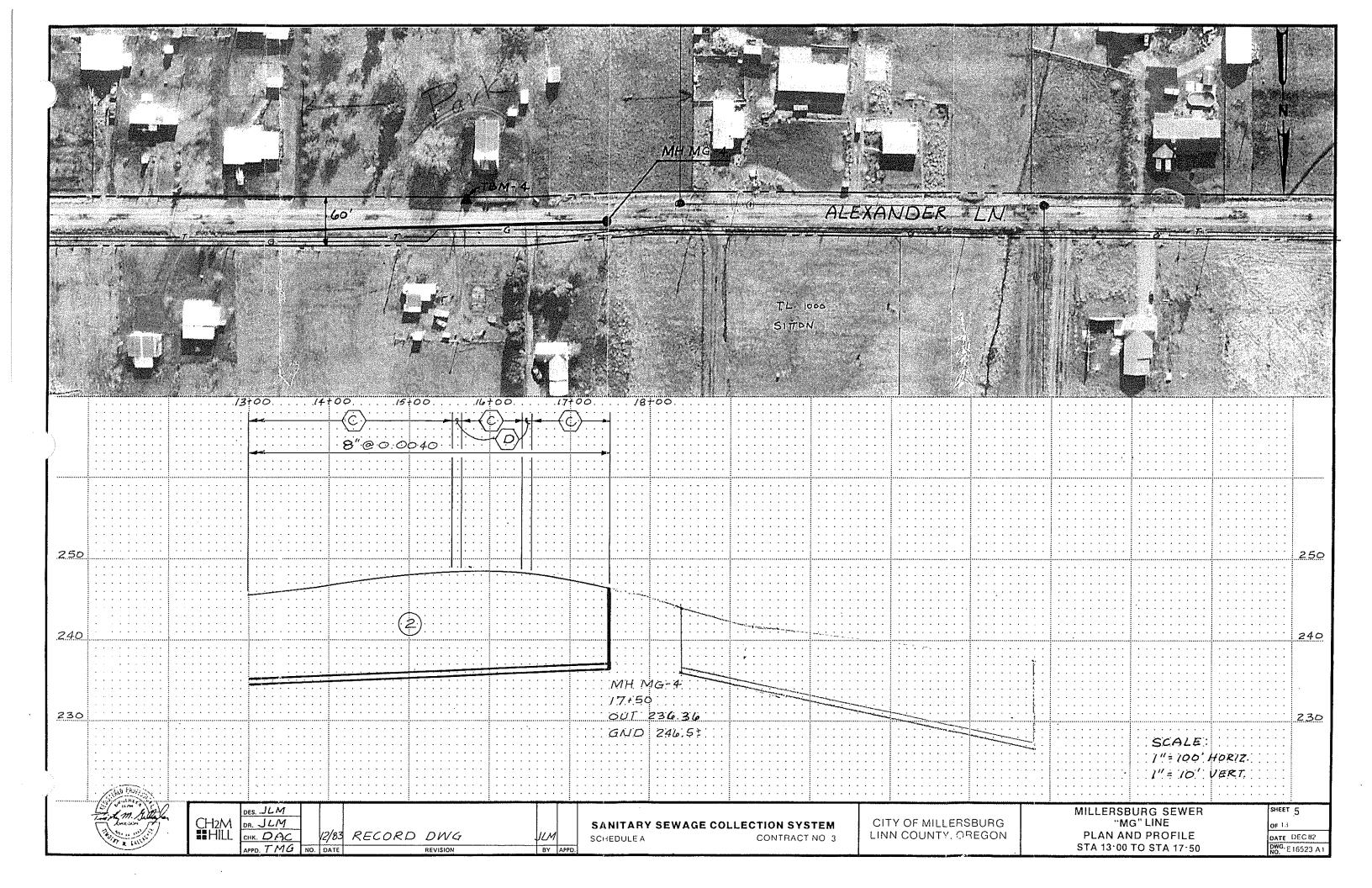
4. City of Millersburg Sanitary Sewer Collection System Contract 3

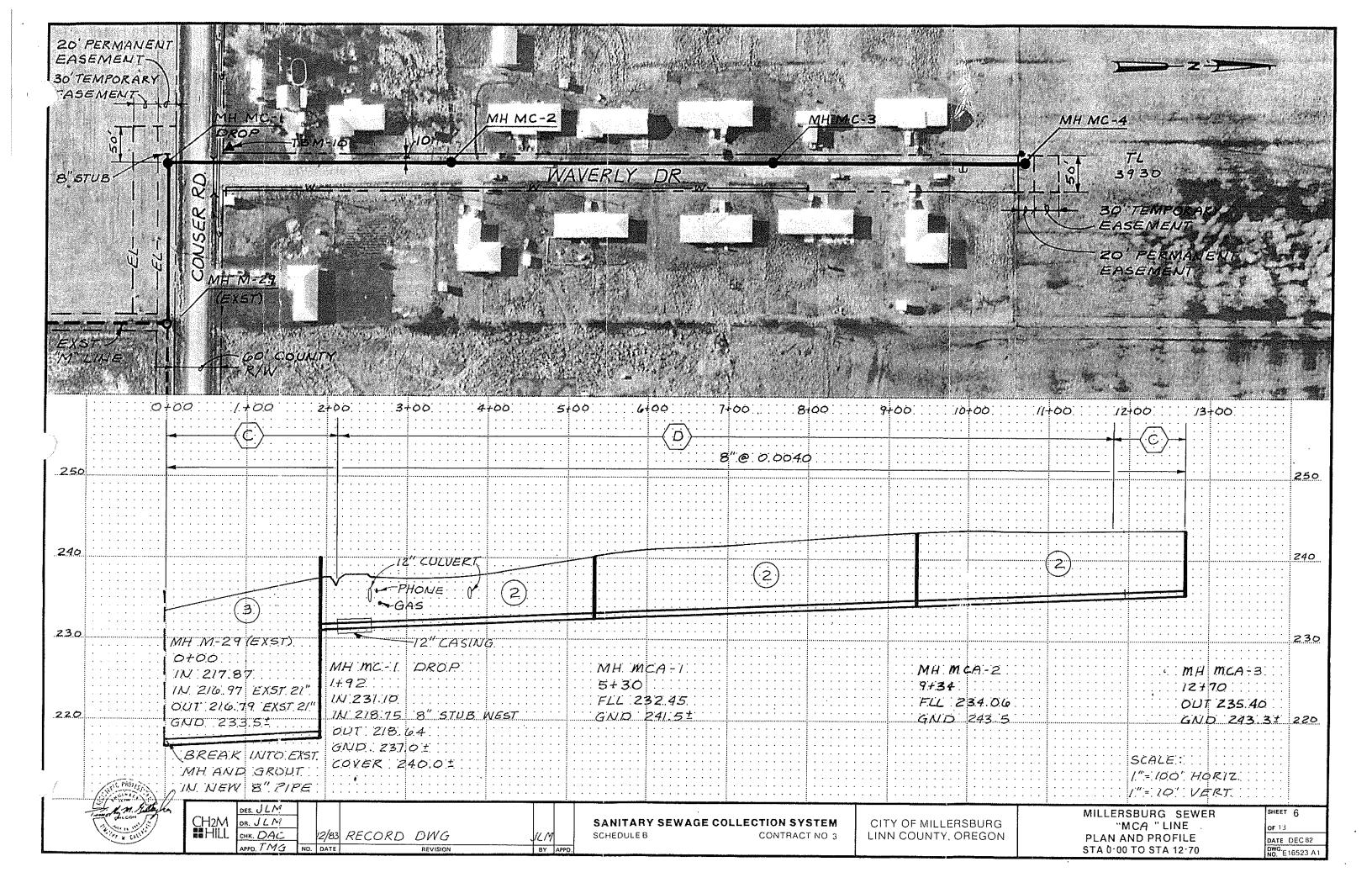


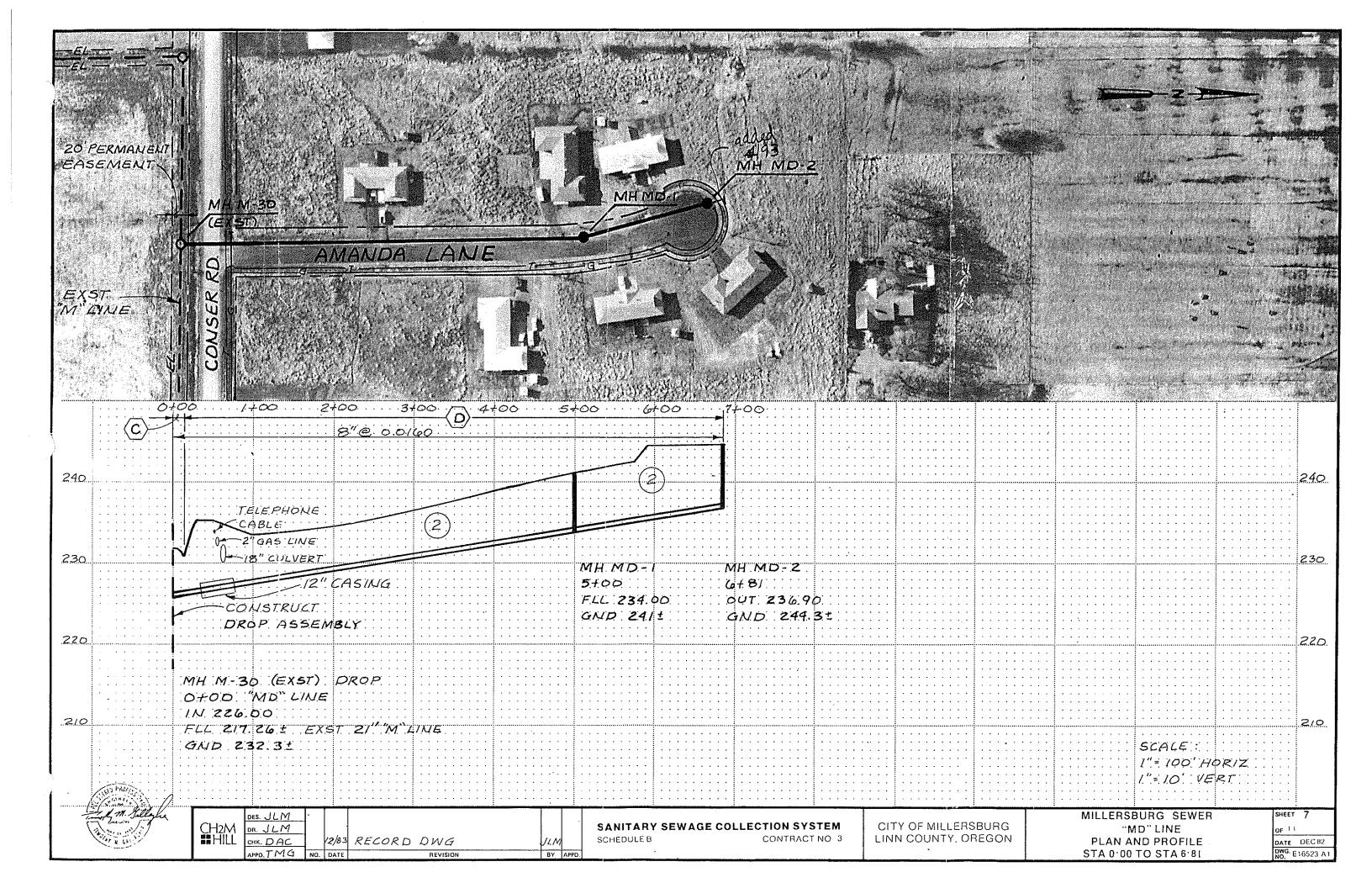


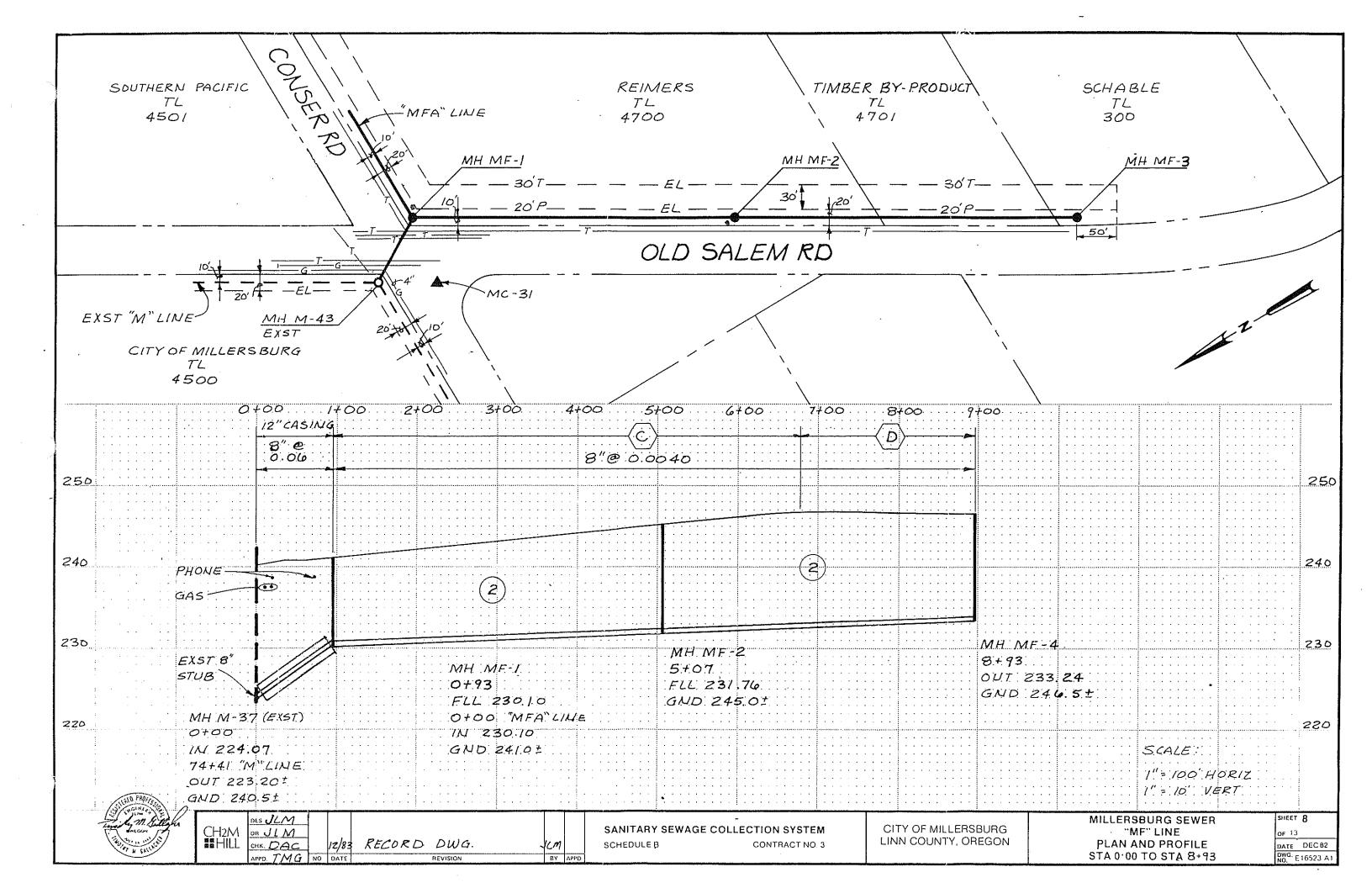


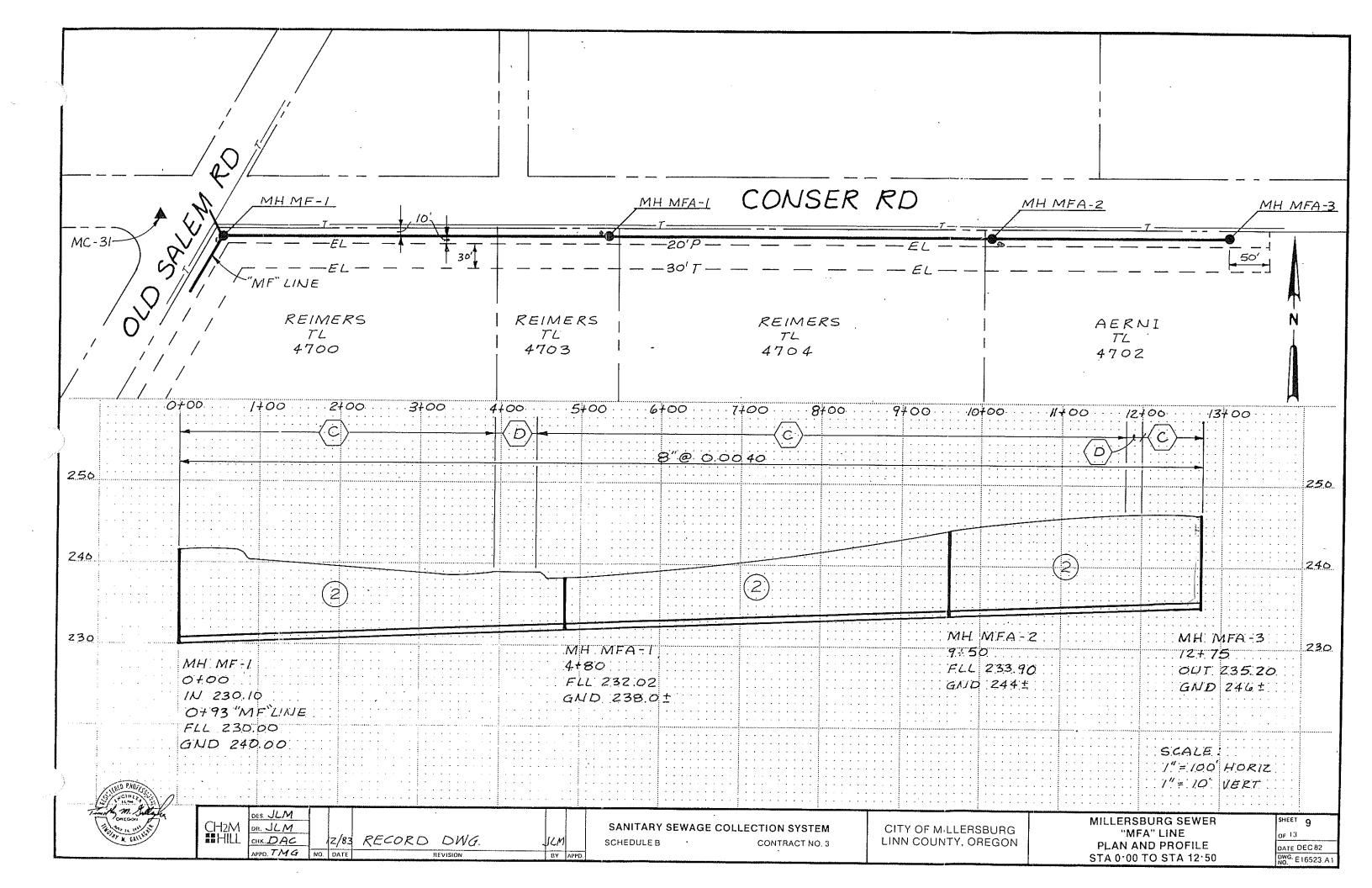




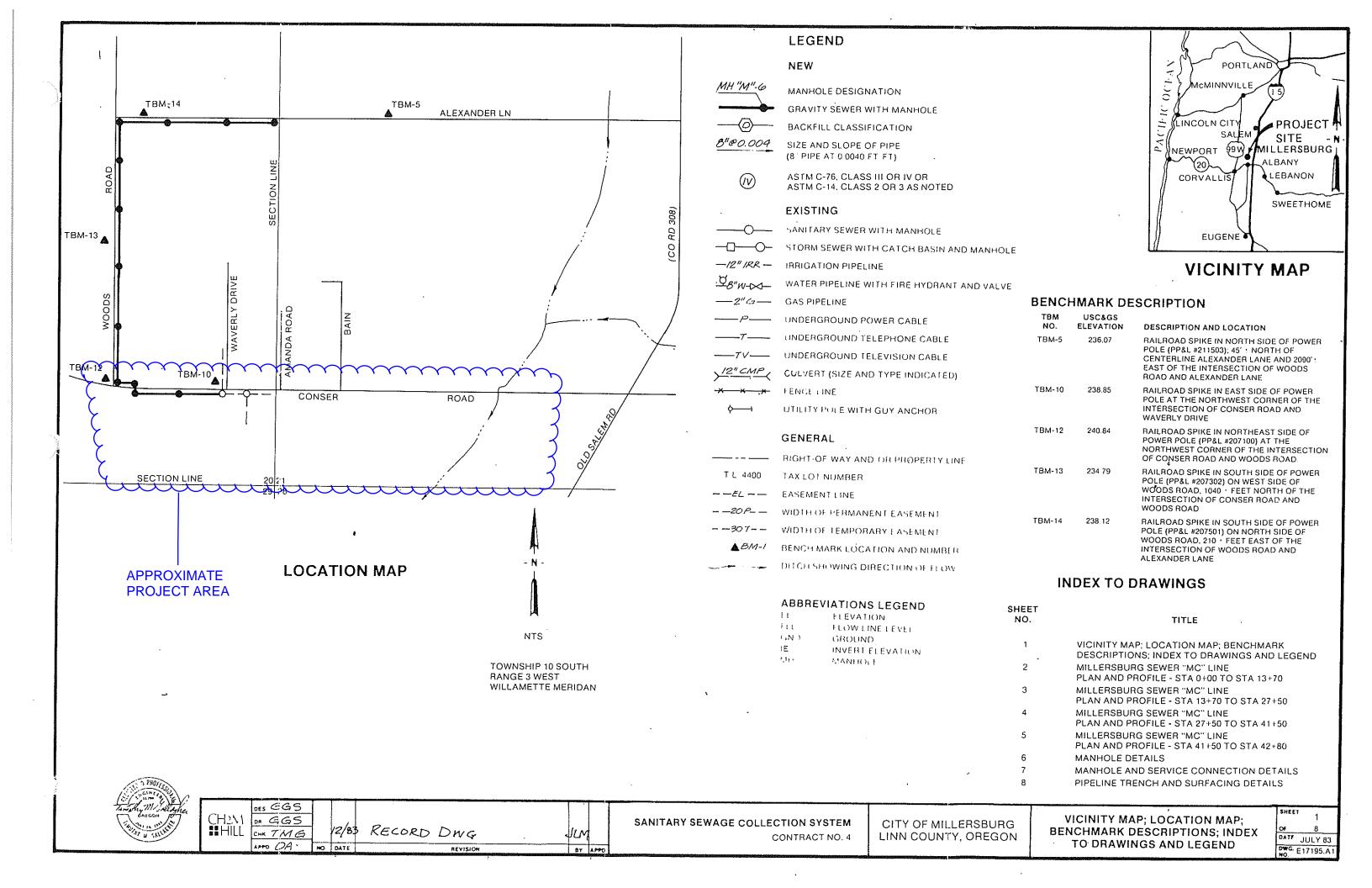


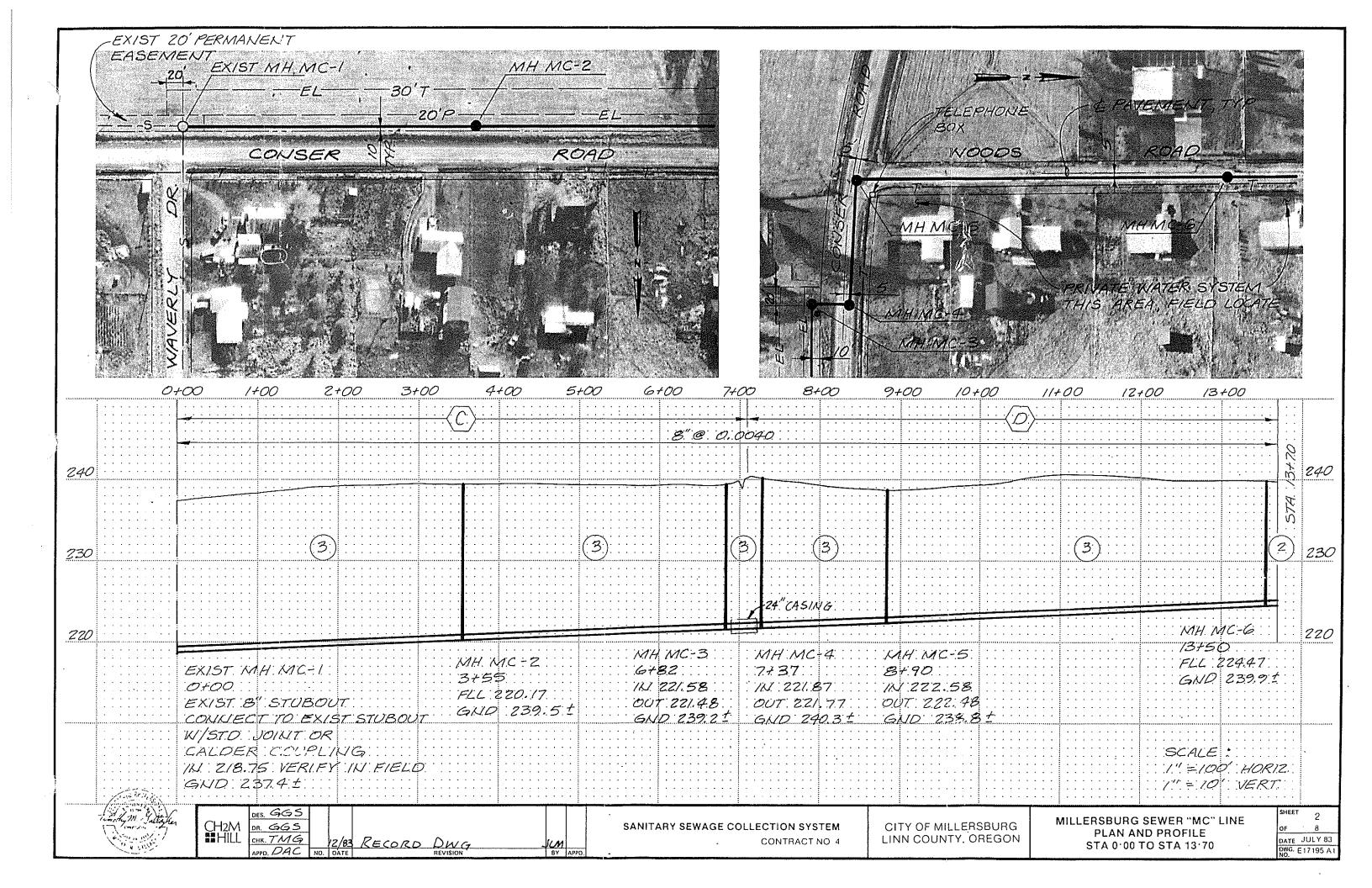


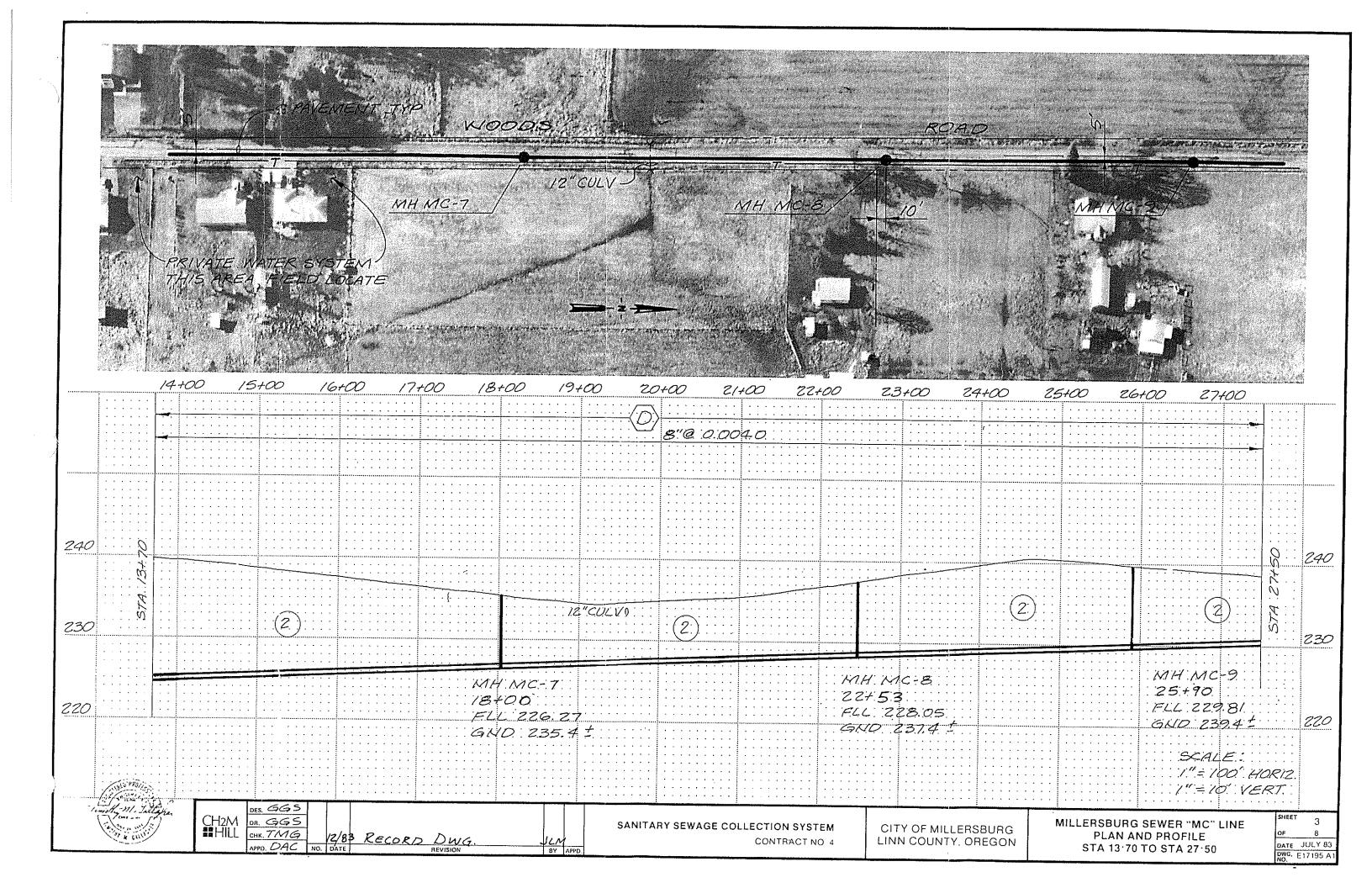


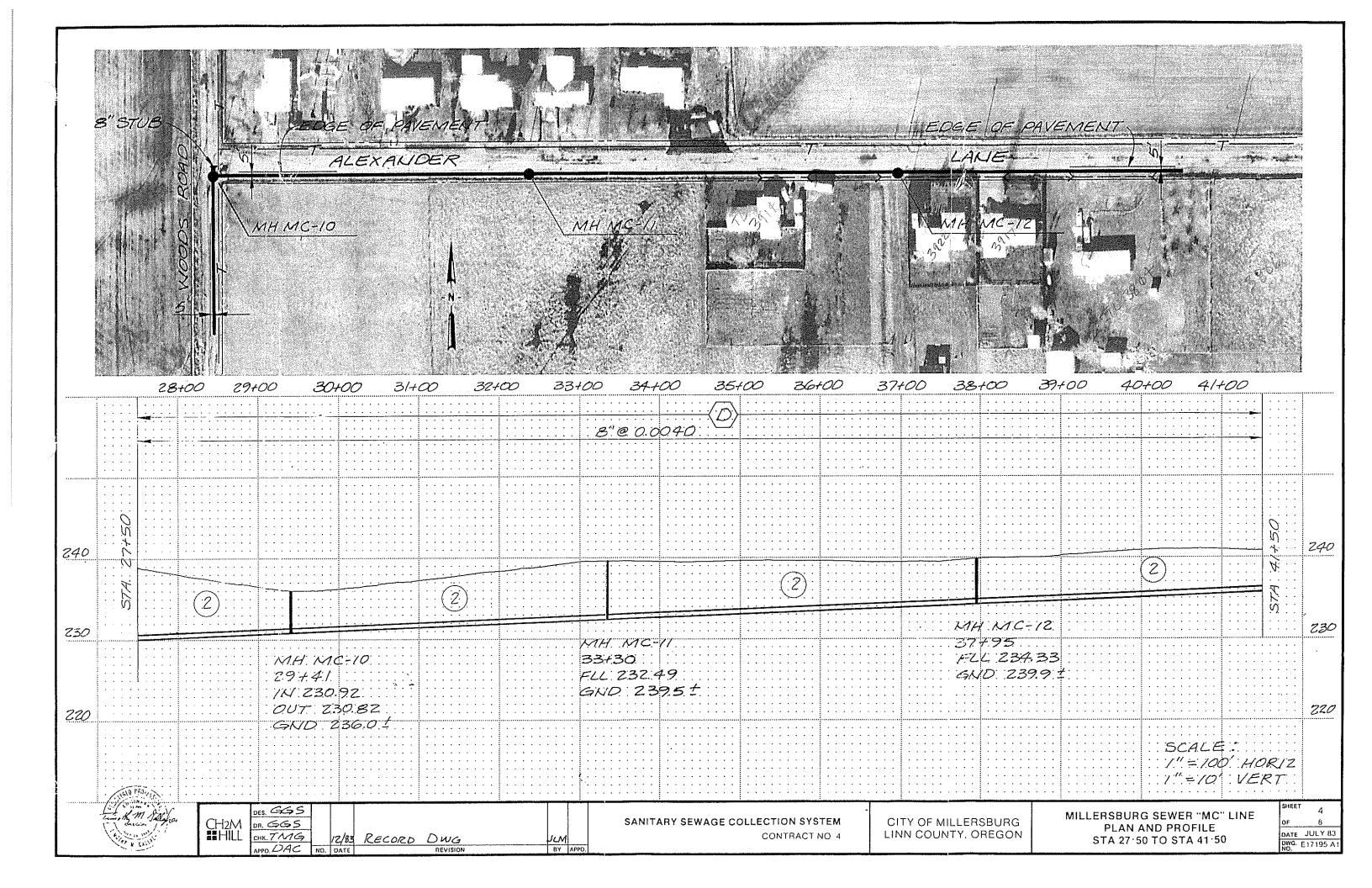


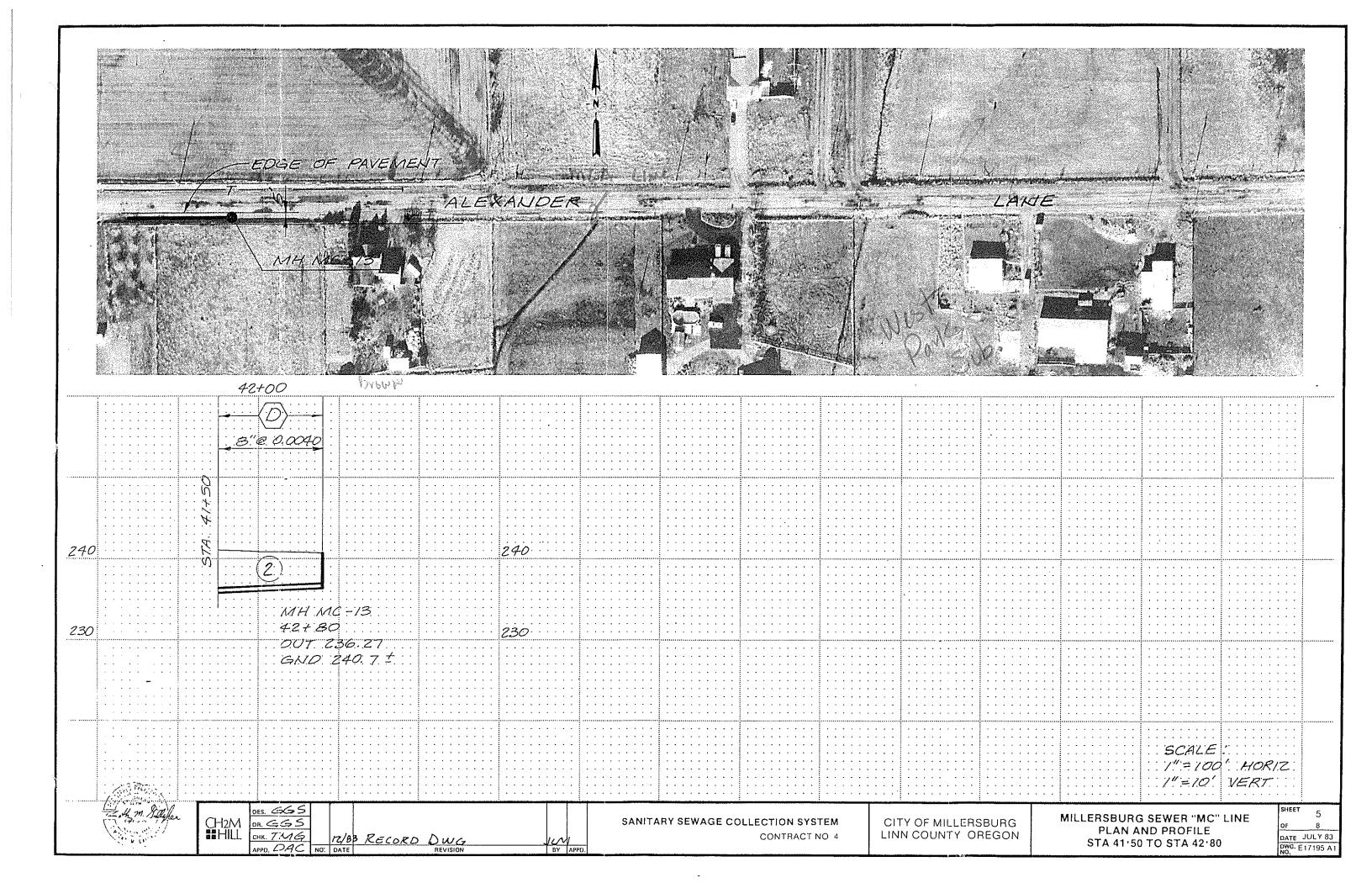
5. City of Millersburg Sanitary Sewer Collection System Contract 4



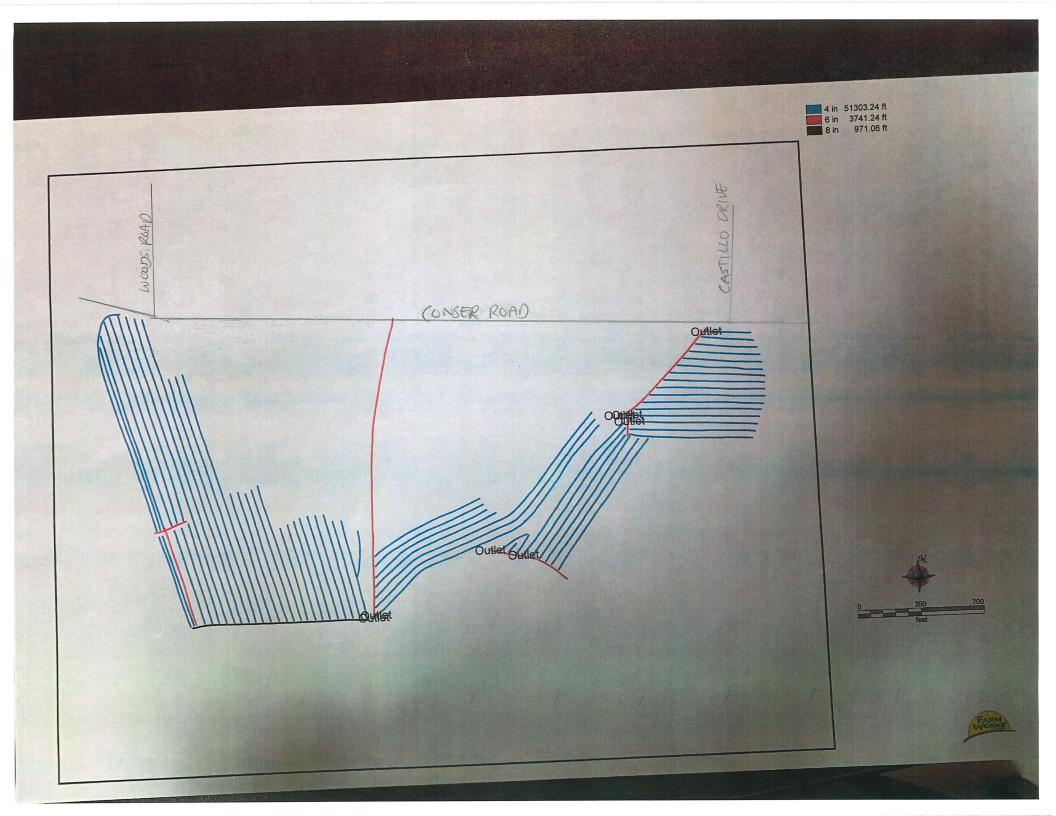








6. Drain Tile Figure with Streets Added



7. Millersburg Fire Station 15 Conformed Civil Drawings





CONCEPTUAL RENDERING

Station 15 Millersburg Fire

3215 Transition Parkway NE Albany, OR 97321

CONFORM SET
4/2/21
Soderstrom
Architects

DEFERRED SUBMITTALS

REFER TO SPECIFICATION SECTION 01 3000 AND 01 4000 FOR BIDDER DESIGN REQUIREMENTS FOR BOTH AHJ REVIEW ITEMS AND NON-AHJ DEFFERRED ITEMS. SUBMITTAL DOCUMENTS FOR AHJ DEFERRED SUBMITAL ITEMS SHALL BE SUBMITED TO THE ARCHITECT OF RECORD BY THE GENERAL CONTRACTOR. ARCHITECT AND APPROPRIATE ENGINEER OF RECORD SHALL REVIEW AND RETURN. THE GENERAL CONTRACTOR SHALL THEN FORWARD AHJ SUBMITTAL ITEMS TO THE BUILDING OFFICAL FOR AHJ APPROVAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND THAT THEY HAVE BEEN FOUND IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE AHJ DEFERRED SUBMITAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL

AUTHORITY HAVING JURISDICTION (AHJ) DEFFERRED SUBMITTAL ITEMS:

- A. PRECAST ARCHITECTURAL CONCRETE AND ATTACHMENTS, DIVISION 03
- FIRE SPRINKLER SYSTEM, DIVISION 21
- FIRE ALARM, DIVISION 28
- STRUCTURAL ENGINEERING FOR MECHANICAL SYSTEM SEISMIC RESTRAINTS
- STRUCTURAL ENGINEERING FOR OPEN WEB JOISTS, DIVISION 6
- LANDSCAPE IRRIGATION DESIGN

SEE SPECIFICATION SECTION 01 4000 SUBMITTAL REQUIREMENTS FOR NON-AHJ BIDDER DESIGNED/ENGINEERED ITEMS.

Station 15

PROJECT ADDRESS:

3215 Transition Parkway NE Albany, OR 97321

PROJECT SUMMARY:

CONSTRUCTION OF A NEW SINGLE-STORY FIRE STATION WITH ROOM FOR 5 APPARATUS AND DORMATORIES FOR SIX ALONG WITH A COMMUNITY MEETING ROOM.

PROJECT TEAM

OWNER CITY OF MILLERSBURG cityofmillersburg.org 4222 NE Old Salem Road Millersburg, OR 97321 (458) 233-6300 Kevin Kreitman, City Manager Janelle Booth, Assistant City Manager

<u>ARCHITECT</u> SODERSTROM ARCHITECTS, LTD. www.sdra.com 1200 NW Natio Parkway, Suite 410 Portland, OR 97209 (503) 228-5617 Fax. (503) 273-8584 Dan VanCalcar, Principal Erica Jankowski, Project Manager/Architect

STRUCTURAL ENGINEER CROW ENGINEERING www.crowengineering.com 9925 SW Nimbus Ave, Suite 110 Beaverton, OR 97008 (503) 213-2013 Joe Kurth, PE, Vice President/Director of Engineering

MECHANICAL / ELECTRICAL / PLUMBING ENGINEERS **CORBIN CONSULTING ENGINEERS** www.corbinengineering.com 1905 NW 169th PL, Suite 121 Beaverton, OR 97006 (503) 645-0176 Mike Machinski, Project Principal

CIVIL ENGINEER CROW ENGINEERING www.crowengineering.com 9925 SW Nimbus Ave, Suite 110 Beaverton, OR 97008 (503) 213-2013

Joe Kurth, PE, Vice President/Director of Engineering

LANDSCAPE ARCHITECT **SODERSTROM ARCHITECTS, LTD.** www.sdra.com 1200 NW Natio Parkway, Suite 410 Portland, OR 97209 (503) 228-5617 Fax. (503) 273-8584 Laurence Ferar, PLA

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INTERIOR DETAILS - CEILINGS

INTERIOR DETAILS - CEILINGS

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FRAMING DETAILS

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E4.01	ENLARGED ELECTRICAL PLAN				
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E5.02	ELECTRICAL DETAILS				
E6.01	ELECTRICAL ONE-LINE DIAGRAM AND GROUNDING RISER DIAGRAMS				
E6.02	ELECTRICAL SCHEDULES				
E6.03	ELECTRICAL TAP OUT SYSTEM INTERCONNECTION DIAGRAM				
EL1.01	ELECTRICAL SITE LIGHTING PLAN				
EL1.02	ELECTRICAL LIGHTING GROUND FLOOR PLAN - WEST				
EL1.03	ELECTRICAL LIGHTING CONTROLS PLAN				

ELECTRICAL LOW VOLTAGE GROUND FLOOR PLAN - WEST

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arkway

Transition

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Revisions

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No. Description

1 Pre-Bid Revisions

2/3/21

Station

CONFORM SET

4/2/21

Project Number 20006

Drawing Title

PROJECT INFO

Sheet No

2. THE CONSTRUCTION CONTRACT IS FOR THE CONSTRUCTION OF A COMPLETE AND FULLY FUNCTIONING INSTALLATION. THESE DOCUMENTS DESCRIBE THE DESIGN INTENT AND SPECIFIC REQUIREMENTS OF THE INSTALLATION. THESE DOCUMENTS DO NOT INTEND TO SHOW EVERY ITEM REQUIRED TO CONSTRUCT THE WORK. ITEMS SUCH AS FASTENERS. CONNECTORS. FILLERS MISCELLANEOUS CLOSURE ELEMENTS, ANCILLARY CONTROL WIRING AND POWER WHERE REQUIRED FOR THE CONTROL OR OPERATION OF THE PROVIDED EQUIPMENT ARE NOT ALWAYS SHOWN BUT ARE CONSIDERED INCLUDED IN THE SCOPE OF THE WORK. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE A FULLY FUNCTIONING INSTALLATION WHICH MEETS THE DESIGN INTENT, INCLUDING THE SPECIFIC REQUIREMENTS INCLUDED IN THESE DOCUMENTS.

ALL ITEMS IN THESE DOCUMENTS ARE NEW UNLESS OTHERWISE NOTED.

THESE DOCUMENTS DESCRIBE A SINGLE CONSTRUCTION CONTRACT. THE USE OF SUBCONTRACTORS IS THE ELECTION OF THE CONTRACTOR. THESE DOCUMENTS DO NOT INTEND TO DIVIDE THE WORK AMONG THE CONTRACTOR'S SUBCONTRACTORS. WHERE THE DOCUMENTS IDENTIFY WORK WHICH IS "NOT IN MECHANICAL WORK" OR "NOT IN ELECTRICAL WORK" IT MEANS THAT WORK IS NOT FURTHER DESCRIBED OR SPECIFIED IN THE MECHANICAL OR ELECTRICAL DRAWINGS OR SPECIFICATIONS. IT DOES NOT PRECLUDE THE CONTRACTOR FROM DELEGATING THE WORK TO THE ENTITIES OF HIS ELECTION. IN ADDITION THE DIVISION OF THE CONTRACT DOCUMENTS INTO ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL AND OTHER DESIGN DISCIPLINES NEITHER DIVIDES THE WORK FOR THOSE DISCIPLINES AS SHOWN ONLY IN THOSE DRAWINGS OR SPECIFICATIONS.

ITEMS INDICATED IN THIS SET NOTED "BY OWNER" ARE NOT IN THE CONTRACT (N.I.C.)

UNLESS OTHERWISE NOTED, IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THE SUBCONTRACTORS TO REVIEW ALL DRAWINGS, PROJECT MANUAL, ADDENDA, ETC. IN ORDER TO ASSURE THE COORDINATION OF ALL WORK AFFECTING EACH TRADE. FAILURE TO REVIEW AND COORDINATE ALL CONTRACT DOCUMENTS BY THE GENERAL CONTRACTOR WITH ALL THE SUBCONTRACTORS FOR APPLICABLE ITEMS OF THE WORK SHALL NOT RELIVE THE RESPONSIBLE PARTY FROM PERFORMING ALL WORK SO REQUIRED AS PART OF THE CONTRACT

UNLESS OTHERWISE NOTED. THE PROJECT MANUAL. WHICH INCLUDES THE GENERAL CONDITIONS. SUPPLEMENTAL CONDITIONS, AND TECHNICAL SPECIFICATIONS, AND THE DRAWINGS ARE COMPLEMENTARY AND TOGETHER DESCRIBE THE PROJECT REQUIREMENTS. WHERE THERE ARE DISCREPANCIES BETWEEN THE PROJECT MANUAL AND THE DRAWINGS, THE CONTRACTOR SHALL ADVISE THE ARCHITECT AND REQUEST A CLARIFICATION. THE ORDER OF PRECEDENCE BETWEEN THE DRAWINGS AND THE PROJECT MANUAL IS AS DEFINED IN THE PROJECT MANUAL

UNLESS OTHERWISE NOTED, THE CONTRACTOR SHALL LAYOUT AND SEQUENCE THE INSTALLATION OF THE WORK SO THAT THE DIFFERENT SYSTEMS DO NOT OBSTRUCT THE INSTALLATION OF SUCCESSIVE WORK. IN GENERAL, SYSTEMS INSTALLED FIRST SHOULD BE KEPT AS HIGH AND TIGHT TO STRUCTURE AS POSSIBLE TO LEAVE SPACE AVAILABLE FOR SYSTEMS WHICH **FOLLOW**

REFER TO THE PROJECT MANUAL FOR SPECIFICATIONS, GENERAL INFORMATION, PRODUCTS AND EXECUTION REQUIREMENTS. REQUIREMENTS OF THE SPECIFICATIONS APPLY TO ALL ASPECTS OF THE WORK AND ARE INCLUDED AS ADDITIONAL INFORMATION FOR EACH ITEM SPECIFIED. IF DISCREPANCIES EXISTS BETWEEN THE SPECIFICATIONS AND DRAWINGS, THE MORE STRINGENT REQUIREMENTS SHALL PREVAIL. THE GENERAL CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVES OF ANY DISCREPANCIES

10. THE DRAWINGS SHALL NOT BE SCALED. THE GENERAL CONTRACTOR SHALL REFER TO THE DIMENSIONS INDICATED OR THE ACTUAL SIZES OF CONSTRUCTION ITEMS. WHERE NO DIMENSIONS OR METHOD OF DETERMINING A LOCATION IS GIVEN, VERIFY CORRECT DIMENSIONS OR LOCATION WITH THE OWNER'S REPRESENTATIVE PRIOR TO INSTALLATION.

11. THE DRAWINGS AND REFERENCED DETAILS HAVE BEEN DIMENSIONED IN ORDER TO ESTABLISH THE CONTROL AND GUIDELINES FOR FIELD LAYOUT. WHERE A DISCREPANCY EXISTS BETWEEN THE DRAWING AND THE DETAIL THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE FOR CLARIFICATION PRIOR TO INSTALLATION.

12. DIMENSIONS ARE TO FACE OF STUD UNLESS OTHERWISE NOTED

13. WHERE DIMENSIONS ARE NOTED TO BE VERIFIED IN THE FIELD (VIF) THE DIMENSION SHOWN IS THE DESIGN BASIS, BUT MAY DIFFER FROM ACTUAL CONDITIONS. CONTRACTOR SHALL VERIFY THESE DIMENSIONS WHILE LAYING OUT THE WORK AND REPORT ANY DISCREPANCIES BETWEEN THE DESIGN BASIS AND ACTUAL DIMENSIONS TO THE OWNER'S REPRESENTATIVE PRIOR TO PROCEEDING WITH THE WORK. WHERE DIMENSIONS ARE NOTED "+/-" FIELD DIMENSIONS MAY VARY FROM THE NOTED DIMENSIONS BY MINOR AMOUNTS. IF THE CONTRACTOR IDENTIFIES DIMENSIONS IN THE FIELD THAT DIFFER BY MORE THAN 1" FROM THE +/- DIMENSIONS INDICTED IN THE DRAWINGS, THE CONTRACTOR SHOULD CONFIRM DIFFERENTIAL WITH ARCHITECTS

14. INTERIOR DETAILS ARE KEYED TO THE PLANS AT TYPICAL LOCATIONS. TYPICAL DETAILS APPLY TO ALL LOCATIONS WHICH ARE SIMILAR BUT ARE NOT OTHERWISE DETAILED. THE CONTRACTOR AND SUBCONTRACTORS ARE RESPONSIBLE TO COORDINATE THE LOCATION OF TYPICAL DETAILS AND INSTALL THE WORK INDICATED. IF DISCREPANCIES EXIST OR QUALIFICATION IS REQUIRED, THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE FOR CLARIFICATION PRIOR TO PROCEEDING.

15. INTERIOR FINISHES ARE KEYED TO THE DRAWINGS AT TYPICAL LOCATIONS. THE FINISHES APPLY TO ALL LOCATIONS WHICH ARE SIMILAR BUT ARE NOT OTHERWISE DETAILED. CONTRACTOR AND SUBCONTRACTORS ARE RESPONSIBLE TO COORDINATE THE LOCATION ALL TYPICAL DETAILS AND INSTALL THE WORK INDICATED. IF DISCREPANCIES EXIST OR QUALIFICATION IS REQUIRED, THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE FOR CLARIFICATION PRIOR TO PROCEEDING.

16. ABBREVIATIONS ON THIS SHEET APPLY TO THE ENTIRE SET UNLESS OTHERWISE NOTED.

17. WALL FIRE RATING INDICATIONS ON THE FLOOR PLANS SHOW EXTENT OF FIRE RATED PARTITION. FIRE RATING IN A PARTITION SHALL CONTINUE OVER DOOR OR WINDOW OPENING WHETHER OR NOT THEY APPEAR IN PLAN.

18. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO VERIFY SIZE AND INVERT ELEVATION OF OPENINGS / SLEEVES THROUGH CONCRETE AND MASONRY WALLS AND CONCRETE FOUNDATION WALLS. OPENINGS / SLEEVES ARE NOT LIMITED TO THOSE SHOWN ON STRUCTURAL DRAWING SHEETS.

19. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO COORDINATE AND MAKE PROVISIONS FOR ALL PIPE / CONDUIT SLEEVES THROUGH CONCRETE WALLS.

20. ELEVATIONS ARE TO TOP OF CONCRETE OR OTHER HARD SURFACE MATERIAL

21. DETAILS ARE INTENDED TO SHOW METHOD AND MANNER OF ACCOMPLISHING THE WORK. MINOR MODIFICATIONS MAY BE REQUIRED TO SUIT JOB DIMENSIONS OR CONDITIONS AND SHALL BE INCLUDED AS PART OF THE WORK.

22. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS AND EXISTING CONDITIONS AT THE SITE BEFORE COMMENCING WORK AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO START OF THE WORK. IN CASE OF CONFLICT BETWEEN ARCHITECTURAL AND CONSULTANTS DRAWINGS, THE ARCHITECT WILL DETERMINE THE CORRECT INTENTION OF THE WORK

23. THE BUILDING SHALL BE PROVIDED WITH A FULL SPRINKLER SYSTEM COMPLYING WITH APPLICABLE CODES OF THE AUTHORITY HAVING JURISDICTION.

24. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES, SYMBOLS, AND TYPICAL DETAILS. SPECIFIC NOTES ON DETAILS APPLY TO SIMILAR CONDITIONS UNLESS NOTED OTHERWISE (UNO / UON).

25. WHERE FIRE RATED OPENING PROTECTION IS REQUIRED, THE FIRE DOORS AND SMOKE AND DRAFT CONTROL ASSEMBLIES INSTALLED IN CORRIDOR OPENINGS SHALL BE TESTED AND LABELED IN ACCORDANCE WITH OSSC CURRENT EDITION SECTION 714. IN ACCORDANCE WITH THE REQUIREMENTS OF THE LISTED ASSEMBLY. THE MANUFACTURER'S INSTALLATION INSTRUCTIONS SHALL BE PROVIDED WITH EACH ASSEMBLY FOR INSTALLATION AND FOR REVIEW BY THE INSPECTION AUTHORITY.

EST

EXH

ESTIMATE

EXPOSED, EXPANSION

EXHAUST

EXTERIOR

EXIST/(E) EXISTING

	ABBREVIATIONS				
•	#	POUND OR NUMBER	FA	FIRE ALARM	
	A/C	AIR CONDITIONING	FAF	FLUID APPLIED FLOORING	
	A/V AB	AUDIO VISUAL ANCHOR BOLT	FD FF(C)	FLOOR DRAIN, FIRE DAMPER	
	AC	ASPHALTIC CONCRETE	FE(C) FF	FIRE EXTINGUISHER (CABINET) FINISH FLOOR	
	ACM	ALUMINUM COMPOSITE METAL	FFE	FURNISHINGS FIXTURES AND	
	ACT	ACOUSTICAL CEILING TILE		EQUIPMENT	
	AD	AREA DRAIN	FGL	FIBERGLASS	
	ADD	ADDENDUM ADHESIVE	FHC FHMS	FIRE HOSE CABINET FLATHEAD MACHINE SCREW	
	ADH ADJ	ADJUSTABLE, ADJACENT	FHWS	FLATHEAD WOOD SCREW	
	AFF	ABOVE FINISH FLOOR	FIN	FINISH(ED)	
	AGG	AGGREGATE	FL	FLOOR	
	AHJ	AUTHORITY HAVING JURISDICTION	FLASHG FLCO	FLASHING FLOOR CLEANOUT	
	AL(UM)	ALUMINUM	FLR	FLOOR(ING)	
	ALT	ALTERNATE	FLS	FIRE LIFE SAFETY	
	ANOD	ANODIZED	FLUOR	FLUORESCENT	
	AP APPROX	ACCESS / ACOUSTIC PANEL APPROXIMATE	FND(N) FOC	FOUNDATION FACE OF CONCRETE	
	ARCH	ARCHITECT(URAL)	FOF	FACE OF FINISH	
	AUTO	AUTOMATIC	FOS	FACE OF STEEL/STUD	
	BATT	BATT INSULATION	FP	FIREPROOFING	
	BD	BOARD	FR	FRAME(D), FRAMING	
	BIT BLDG	BITUMINOUS BUILDING	FS FT	FULL SIZE, FLAME SPREAD FIRE TREATED	
	BLKG	BLOCKING	FTG	FOOTING	
	BM	BENCH MARK	GA	GAUGE	
	ВО	BOTTOM OF	GALV	GALVANIZED	
	BOL	BOLLARD	GB	GRAB BAR, GYPSUM BOARD	
	BOT BR(N)Z	BOTTOM BRONZE	GL GWB	GLASS, GLAZING, GRIDLINE GYPSUM WALL BOARD	
	BSMT	BASEMENT		GYPSUM (BOARD)	
	CAB	CABINET	HB ` ´	HOSE BIB	
	CB	CATCH BASIN	HBD	HARDBOARD	
	CCTV CEM	CLOSED CIRCUIT TV CEMENT	HC HD	HOLLOW CORE HEAVY DUTY	
	CF	CUBIC FOOT	HDR	HEADER	
	CG	CORNER GUARD		HARDWARE	
	CI	CONTINUOUS INSULATION	НМ	HOLLOW METAL	
	CJ(T) CL	CONTROL JOINT CENTERLINE	HOR(IZ) HR	HORIZONTAL HOUR	
	CLG	CEILING	HT	HEIGHT	
	CLR	CLEAR(ANCE)	HTG	HEATING	
	CMU	CONCRETE MASONRY UNIT	HVAC	HEATING, VENTILLATION, AND AIR	
	COL	COLUMN	HWD	CONDITIONING HARDWOOD	
	CONC CONST	CONCRETE CONSTRUCTION	HWH	HOT WATER HEATER	
	CONT		ID	INSIDE DIAMETER	
	COORD	COORDINATE	INCL	INCLUDE(D), INCLUDING	
	CPT	CARPET	INS(UL) INT	INSULATE(D), INSULATION INTERIOR	
	CRS CS	COURSE COUNTERSINK	JAN(T)	JANITOR	
	CSMT	CASEMENT	JC	JANITOR'S CLOSET	
	CT	CERAMIC TILE	JT	JOINT	
	CTR	CENTER	KO	KNOCK-OUT	
	CUST		LAM LAV	LAMINATE(D) LAVATORY	
	CX	CONNECTION CUBIC YARD	LH	LEFT HAND	
	DEM(O)		LW	LIGHTWEIGHT	
	DEP	DEPRESSED	MAX	MAXIMUM	
	DF	DRINKING FOUNTAIN	MB MECH	MACHINE BOLT, MARKER BOARD MECHANICAL	
	DIA DIAG	DIAMETER DIAGONAL	MEZZ	MEZZANINE	
	DIM	DIMENSION	MFR	MANUFACTURE(R)	
	DISP	DISPENSER	MGR	MANAGER	
	DIV	DIVISION	MH	MANHOLE	
	DL	DEAD LOAD	MIN MISC	MINIMUM MISCELLANEOUS	
	DMT DN	DEMOUNTABLE DOWN	MO	MASONRY OPENING	
	DR	DOOR	MOD	MODULAR	
	DS	DOWNSPOUT	MP	METAL PANEL	
	DTL	DETAIL	MRGB	MOISTURE RESISTANT GYPSUM WALL BOARD	
	DWG(S) DWR	DRAWING(S) DRAWER	MTL	METAL	
	E	EAST	MULL	MULLION	
	EA	EACH	MWP	MEMBRANE WATERPROOFING	
	EB	EXPANSION BOLT	N NAT	NORTH NATURAL	
	EJ FLEV	EXPANSION JOINT	NA I NIC	NOT IN CONTRACT	
	EL, ELEV ELEC	ELEVATION ELECTRIC(AL)	NOM	NOMINAL	
	EMER(G)	` ,	NTS	NOT TO SCALE	
	ENCL	ENCLOSE(URE)	OA	OVERALL	
	EOS	EDGE OF SLAB	OC OD	ON CENTER OUTSIDE DIAMETER	
	EPDM	ELECTRICAL PANEL BOARD	OH	OVERHEAD	
	EPDM	ETHYLENE PROPYLENE DIENEMONOMER	OPG	OPENING	
	EQ	EQUAL	OPP	OPPOSITE	
	EQUIP FST	EQUIPMENT ESTIMATE	OVHD	OVERHEAD	
	EST	ESTIMATE			

PARAPET

PERFORATE(D)

PLASTIC LAMINATE

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

PRESSURE TREATED, POINT

PAINTED, PAPER TOWEL

POLYVINYL CHLORIDE

REFLECTED CEILING PLAN

REFER(ENCE), REFRIGERATOR

REINFORCE(D), REINFORCING

REVISION(S), REVISED

SELF-ADHERED MEMBRANE

PANEL JOINT

PLATFORM

DISPENSER

PARTIAL

PARTION

PLYWOOD

QUARRY TILE

RETURN AIR

ROOF DRAIN

REQUIRED

RIGHT HAND

ROUGH OPENING

ROOM

ROUND

SOUTH

SUPPLY AIR

SOLID CORE

SCHEDULE

SECTION

SIMILAR

SLEEVE

SQUARE

STEEL

STANDARD

STRUCTURAL

SUSPENDED

SHEET VINYI

TACKBOARD

TELEPHONE

TOP OF

SPECIFICATION(S)

STAINLESS STEEL

SUPPLEMENT, SUPPLY

TEMPERED, TEMPERATURE

TOP OF CURB, TOP OF CONCRETE

TONGUE AND GROOVE

TOP OF FRAMING

TOP OF PARAPET

TOP OF PLATE

TOP OF ROOF

TOP OF STEEL

TOP OF WALL

UNDERCOUNTER

UON, UNO UNLESS NOTED OTHERWISE

VAPOR BARRIER

VERIFY IN FIELD

WATER CLOSET

WATERSTOP

PLUS OR MINUS

DIAMETER

WATERPROOF(ING)

WELDED WIRE FABRIC

EXTRUDED POLYSTYRENE

VINYL COMPOSITION TILE

TUBE STEEL

TYPICAL

VERTICAL

VERIFY

WEST WITH

WITHOUT

WOOD

YARD

VESTIBULE

RESILIENT BASE

RADIUS

PANEL

PAINT

PAR

PLAT

PNL

PNT

PTL

PTN

PVC

PWD

RAD

REINF

REQ(D)

REV

RM

RND

RO

SAM

SCHED

SEC(T)

SPEC(S)

STRUCT

TCKBD

TEL

TO

TOF

TOP

TOR

U/C

VCT

VERT

VEST

VFY

W/O

WD

XPS

ΥD

TEMP

TG, T&G

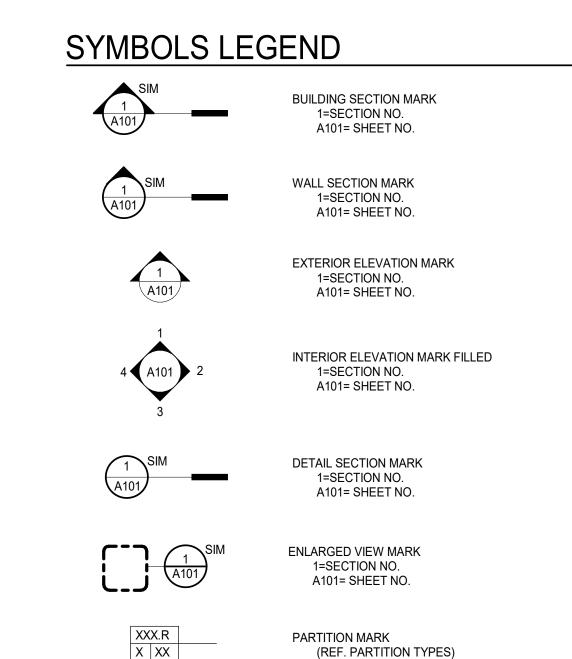
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STD

STL





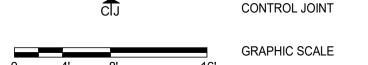
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A1001













MATERIALS LEGEND

CONCRETE CONCRETE

BRICK (PLAN/SECTION)	UNDISTURBED EARTH
BRICK (ELEVATION)	DISTURBED EARTH
STUCCO	STEEL
RIGID INSULATION	GYPSUM BOARD
SPRAY INSULATION	DRAINAGE FILL
CONCRETE MASONRY	PLYWOOD
[4 /	*******

BLANKET OR LOOSE FILL INSULATION

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Revisions

No. Description



CONFORM SET

4/2/21 Project Number 20006

NOTES, SYMBOLS, **LEGENDS, AND ABBREVIATIONS**

G2.01

Drawing Title

SHEET NOTES

 ARCHITECTURAL SITE PLAN SHOWN FOR REFERENCE ONLY. REFER TO CIVIL AND LANDSCAPE DRAWINGS FOR SPECIFIC SITE INFORMATION.
 SEE SHEET A8.50 FOR SIGNAGE DETAILS Soderstrom Architects

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CITY OF A COMMUNITY LINKING AGRICULTURE AND INDUSTRY

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2 Addendum #2

Date 3/2/21

Stamp



CONFORM SET

Date

4/2/21

Project Number 2006

Drawing Title

ARCHITECTURAL

SITE PLAN

Sheet No **A1.01**

OVERALL SITE PLAN

1/32" = 1'-0"

GENERAL CIVIL NOTES:

COURSE OF CONSTRUCTION.

- 1. ALL WORK & MATERIALS SHALL CONFORM TO THE 2017 OREGON PLUMBING SPECIALTY CODE & ALL APPLICABLE STATE, CITY, AND COUNTY REGULATIONS AND STANDARDS. CONTACT ENGINEER FOR DIRECTIVE IN THE EVENT OF CONFLICTING STANDARDS.
- 2. CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE CURRENT EDITION OF THE CITY OF ALBANY STANDARD CONSTRUCTION SPECIFICATIONS (AS ADOPTED BY THE CITY OF MILLERSBURG), THE PROJECT SPECIFICATIONS, CONSTRUCTIONS DRAWINGS, AND THESE SPECIAL PROVISIONS. IN SITUATIONS WHERE SPECIFICATION REQUIREMENTS DIFFER, THE MORE STRINGENT REQUIREMENT SHALL APPLY.
- 3. ALL WORK WITHIN THE PUBLIC RIGHT-OF-WAY SHALL BE COORDINATED WITH THE GOVERNING AGENCY'S INSPECTOR AND SHALL CONFORM TO THAT AGENCY'S CURRENT ENGINEERING STANDARD SPECIFICATIONS & DETAILS.
- 4. THE GENERAL CONTRACTOR AND ALL THEIR AFFILIATES SHALL VERIFY ALL DIMENSIONS, ELEVATIONS & LOCATIONS PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- EXISTING SITE SURVEY PROVIDED BY S&F LAND SERVICES, DATED MAY 7, 2020. ELEVATIONS ARE BASED ON NGVD GPS TIES TO LINN COUNTY SURVEY BENCHMARKS 93192, 93014, AND 93012, HORIZONTAL DATUM BASED IN THE OREGON NORTH STATE PLAN COORDINATE SYSTEM NAD 83 (2011).
- 6. ALL GRADE SURVEYING AND HORIZONTAL LAYOUT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR & SHALL BE PERFORMED BY A LICENSED LAND SURVEYOR. COORDINATE WITH ENGINEER PRIOR TO CONSTRUCTION.
- 7. ALL EXISTING SITE UTILITIES IDENTIFIED ON THIS PLAN ARE NOT INTENDED TO BE EXACT OR COMPLETE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO IDENTIFY ALL UTILITIES AND PROTECT AS REQUIRED DURING THE
- CONTRACTOR SHALL NOTIFY ALL APPLICABLE REGULATORY AGENCIES AND UTILITY COMPANIES 48 hrs PRIOR TO BEGINNING WORK.
- 9. ALL SITE EXCAVATION, TRENCH BACK FILL, PARKING LOT SUB-GRADE, FLAT WORK SUB-GRADE, COMPACTION REQUIREMENTS, ETC. SHALL BE AS NOTED IN THE SITE PREPARATION NOTES AND/OR THE GEOTECHNICAL REPORT
- 10. ALL NON-DRIVEABLE SITE CONCRETE SHALL BE f'c = 3500 psi @ 28 DAYS, 6% ENTRAINED AIR, 4" SLUMP (U.N.O.). ALL DRIVEABLE SITE CONCRETE AND CONCRETE WITHIN THE PUBLIC R.O.W. SHALL BE fc = 4000 psi PER THE CITY OF ALBANY STANDARD CONSTRUCTION SPECS.
- 11. ALL UTILITY SERVICES SHALL BE INSTALLED PER THE RESPECTIVE UTILITY CODES & STANDARDS.
- 12. WATER MAINS WITHIN THE PUBLIC R.O.W. SHALL HAVE A MINIMUM COVER OF 36". ALL OTHER UTILITIES SHALL HAVE A MINIMUM COVER OF 30" UNLESS OTHERWISE SPECIFIED.
- 13. ALL SERVICES SHALL BE ADEQUATELY MARKED AS REQ'D AS TO IDENTIFY THE SIZE, TYPE, & DEPTH OF THE SERVICE.
- 14. ALL SERVICES SHALL BE PLUGGED AS REQ'D TO ADEQUATELY ENSURE THAT NO FOREIGN MATERIALS ENTER THE
- 15. CONTRACTOR SHALL PROVIDE THE ENGINEER WITH THE SIZE, TYPE, DEPTH OF MAIN, TYPE OF CONNECTION AT MAIN, INSTALLATION DATE, LOCATION & SKETCH OF ALL UTILITY SERVICE INSTALLATIONS.
- 16. CONTRACTOR SHALL OBTAIN ALL APPLICABLE PERMITS PRIOR TO CONSTRUCTION.
- 17. ALL FIRE WATER LINES SHALL BE CLASS 52 DUCTILE IRON PIPE AND ALL DOMESTIC WATER LINES SHALL BE PVC WATER PIPE CONFORMING TO ASTM D 1785 WITH SOLVENT-CEMENTED JOINTS.
- 18. ALL SANITARY SEWER WASTE LINES SHOWN OUTSIDE THE BUILDING SHALL BE PVC SEWER PIPE CONFORMING TO ASTM D 3034 - SDR 35 WITH GASKET JOINTS. SEE MECHANICAL PLANS FOR ALL PIPING REQUIREMENTS WITHIN 5' OF
- 19. SANITARY LINES SHALL BE REQ'D TO PASS A LOW PRESSURE AIR TEST OR WATER TEST CONFORMING TO PLUMBING CODE SPECIFICATIONS PRIOR TO FINAL ACCEPTANCE. ALL PARTS OF THE SYSTEM SHALL BE CLEANED PRIOR TO FINAL ACCEPTANCE. THE CONTRACTOR SHALL NOT ALLOW ANY FOREIGN MATERIAL TO ENTER THE EXISTING SYSTEM. THE CONTRACTOR SHALL PROVIDE THE REQ'D PERSONNEL AND MATERIAL TO PERFORM THE ABOVE TESTS. THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH DOCUMENTATION OF THE ABOVE TESTS.
- 20. STORM COLLECTION SYSTEM DESIGNED FOR WATER TIGHT COMPONENTS.
- 21. ALL STORM PIPE IDENTIFIED AS 'PVC' SHALL BE ASTM D 3034 SDR 35. ALL ON-SITE STORM PIPE IDENTIFIED AS 'HDPE' SHALL BE HANCOR SURE-LOK F477 -OR- ADVANCED DRAINAGE SYSTEMS N-12. SEE PLAN SET FOR ADDITIONAL INFORMATION.
- 22. ALL STORM COLLECTION SYSTEM CONNECTIONS AND COMPONENTS SHALL CONFORM TO PIPE MANUFACTURER REQUIREMENTS. GC TO COORDINATE STORM SYSTEM LAYOUT W/ ENGINEER AND STORM SYSTEM SUPPLIER. STORM SYSTEM COMPONENT SHOP DRAWINGS SHALL BE PROVIDED FOR ENGINEER'S REVIEW PRIOR TO CONSTRUCTION.
- 23. ALL ON SITE CATCH BASINS SHALL BE AS IDENTIFIED ON PLAN SET. ALL ON-SITE STORM SYSTEM CATCH BASINS SHALL BE PROVIDED WITH A MINIMUM 24" SETTLEMENT SUMP BELOW THE LOWEST PIPE INVERT (U.N.O.) AND A POLLUTION CONTROL HOOD AND TRAP SYSTEM.SEE PLAN SET FOR ADDITIONAL INFORMATION. CURB INLETS IN ROW PER STANDARD SPECS.
- 24. GC SHALL PROVIDE ENGINEER WITH SHOP DRAWING SUBMITTALS ON ALL PRE-CAST MANUFACTURED ITEMS.
- 25. ALL UNDERGROUND PIPING, CONDUIT AND OTHER UTILITIES SHALL BE BEDDED PER CITY OF ALBANY STANDARD DETAILS (OR AS OTHERWISE SPECIFIED BY PIPE MANUFACTURER). NOTIFY ENGINEER IN EVENT OF DISCREPANCIES.
- 26. ALL TEMPORARY PROTECTION AND DIRECTION OF TRAFFIC SHALL BE BY THE CONTRACTOR AND CONFORM WITH BOTH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES AND THE ODOT MANUAL ON SHORT TERM TRAFFIC CONTROL
- 27. ALL LANDSCAPED AREAS SHALL BE AS NOTED ON THE LANDSCAPE PLANS. THE ENGINEER SHALL INSPECT ALL

LANDSCAPE PLANTER GRADES PRIOR TO RECEIVING FINAL SURFACE TREATMENT.

SITE PREPARATION NOTES:

CLEARING & GRUBBING -

- 1. REFER TO STRUCTURAL (FOUNDATION) PLANS FOR SPECIFIC SOIL EXCAVATION & BACKFILL REQUIREMENTS WITHIN BUILDING FOOTPRINT(S).
- 2. ALL AREAS BELOW ROADWAYS, PARKING AND WALKWAYS SHALL BE CLEARED AND GRUBBED OF ALL PAVEMENT, FOREIGN MATTER, DEBRIS, ORGANIC AND DISTURBED MATERIAL, U.N.O. STRIPPING DEPTHS ACROSS THE SITE WILL VARY DEPENDING ON LOCATION AND PAVEMENT SECTION REQUIREMENTS. ALL EXPOSED MATERIAL SHALL BE MOISTURE CONDITIONED TO WITHIN 2% OF OPTIMUM.
- 3. ALL CLEARED AND GRUBBED MATERIAL SHALL BE REMOVED FROM SITE. GC SHALL COORDINATE AN APPROVED
- 4. ALL AREAS WITH ABANDONED UTILITY LINES, STORM DRAINS, UNDERGROUND TANKS, ETC. WHICH PROVIDE VOID SPACE BENEATH THE SURFACE SHALL BE LOCATED AND REMOVED PRIOR TO SITE GRADING.
- 5. ALL HOLES, DEPRESSIONS, AND UNDISTURBED NATIVE MATERIAL SHALL BE CLEARED OF ALL LOOSE AND ORGANIC MATERIAL THEN BACKFILLED AND COMPACTED WITH APPROVED STRUCTURAL FILL.
- 6. AFTER CLEARING THE ABOVE MENTIONED AREAS, ALL EXPOSED SUB-GRADE SHALL BE PROOF ROLLED WITH A DUMP TRUCK FULLY LOADED WITH ROCK. SOILS SHALL BE REMOVED AND RE-COMPACTED OR REPLACED WITH IMPORTED APPROVED STRUCTURAL FILL IF THEY DO NOT DEMONSTRATE A FIRM, UNYIELDING CONDITION. GEOTECHNICAL ENGINEER SHALL APPROVE SUB-GRADE SURFACE PRIOR TO STRUCTURAL FILL IMPORT EXPLAINED BELOW.

STRUCTURAL FILL PLACEMENT & COMPACTION -

- 7. APPROVED STRUCTURAL FILL SHALL BE PLACED BENEATH AREAS RECEIVING ASPHALT AND/OR CONCRETE
- STRUCTURAL FILL SHALL BE APPROVED BY GEOTECHNICAL ENGINEER PER THE REPORT RECOMMENDATIONS AND CITY OF MILLERSBURG SPECIFICATIONS. ALL FILL SHALL BE FREE OF ORGANIC AND EXPANSIVE CLAY MATERIAL.
- 9. PLACEMENT LIFTS TO BE DETERMINED BY GEOTECHNICAL ENGINEER BASED ON MATERIAL PROPERTIES OF STRUCTURAL FILL CHOSEN AND TYPE OF COMPACTION EQUIPMENT USED. BASE ROCK PLACEMENT LIFTS SHALL NOT EXCEED 8". EACH LIFT SHALL BE NEARLY EQUAL IN THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF ASTM D 1557. FILLS SHALL BE PLACED AT OR SLIGHTLY ABOVE THEIR OPTIMUM MOISTURE CONTENT.
- 10. ALL UTILITY TRENCH BACK FILL SHALL CONFORM TO CURRENT JURISDICTIONAL PUBLIC WORKS SPECIFICATIONS FOR CONSTRUCTION AND THE PROJECT GEOTECHNICAL INVESTIGATION REPORT. CONTACT CIVIL ENGINEER OF RECORD IN THE EVENT OF A CONFLICT.
- 11. IN ADDITION TO THE ABOVE, ALL SITE PREPARATION AND SUBSURFACE WORK SHALL CONFORM TO THE PROJECT GEOTECHNICAL INVESTIGATION REPORT AS PREPARED BY PBS ENGINEERING AND ENVIRONMENTAL, INC.

EROSION CONTROL NOTE:

THE 1200C PLAN SHEETS FOR THIS SITE CONTAIN AN EROSION AND SEDIMENT CONTROL PLAN THAT MUST BE IMPLEMENTED AT THE START OF THIS PROJECT. THE INFORMATION CONTAINED WITHIN THE REFERENCED PLAN SHEETS SHALL BE CONSIDERED A MINIMUM AND SHALL BE MODIFIED AS REQUIRED BY THE CONTRACTOR & CITY INSPECTOR, TO CONTAIN ALL SEDIMENT ON SITE. SPECIAL ATTENTION SHALL BE TAKEN AT ALL EXISTING STORM DRAIN CATCH BASINS AND STORM DRAIN CHANNELS AS TO ELIMINATE ANY SEDIMENT TRANSFER INTO THE EXISTING STORM DRAIN SYSTEM.

AN ALL WEATHER ROCK SURFACE SHALL BE PROVIDED AT ALL CONSTRUCTION SITE ENTRANCES. GC MAY ELECT TO USE (E) GRAVEL PAVING, AC PAVING, ETC. (IF ACCEPTABLE TO CITY INSPECTOR). ALL CONSTRUCTION SHALL BE MAINTAINED WITHIN THE DEVELOPMENT LIMITS OF THIS PHASE. REFER TO EROSION CONTROL SHEET FOR ADDITIONAL INFORMATION.

UTILITY STATEMENT:

EXISTING UNDERGROUND UTILITIES ILLUSTRATED IN THESE PLANS HAVE BEEN LOCATED BY A UTILITY LOCATE COMPANY LAYOUT INDICATED IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. ALL LINES WITHIN PROJECTED WORK ZONE SHALL BE FIELD VERIFIED AS REQ'D PRIOR TO CONSTRUCTION.

ABBREVIATIONS:

ADDREVIATIONS.				
AC	ASPHALT			
APWA	AMERICAN PUBLIC WORKS ASSOCIATION			
ASTM	AMERICAN STANDARD TEST METHOD			
вос	BOTTOM OF CURB			
BOS	BOTTOM OF STAIR			
CMP	CORRUGATED METAL PIPE			
CONC	CONCRETE			
DWG	DRAWING			
(E)	EXISTING			
EG	EXISTING GRADE			
FG	FINISHED GRADE			
GC	GENERAL CONTRACTOR			
GRD	GROUND			
GRVL	GRAVEL			
HDPE	HIGH-DENSITY POLYETHYLENE			
IE	INVERT ELEVATION			
MAX	MAXIMUM			
MIN	MINIMUM			
NGVD	NORTH GEODETIC VERTICAL DATUM			
REQD	REQUIRED			
ROW	RIGHT-OF-WAY			
STD	STANDARD			

TOP OF BASEROCK LAYER

TOP OF SUBGRADE LAYER

TOP BACK OF CURB

TOP OF STAIR

TYPICAL

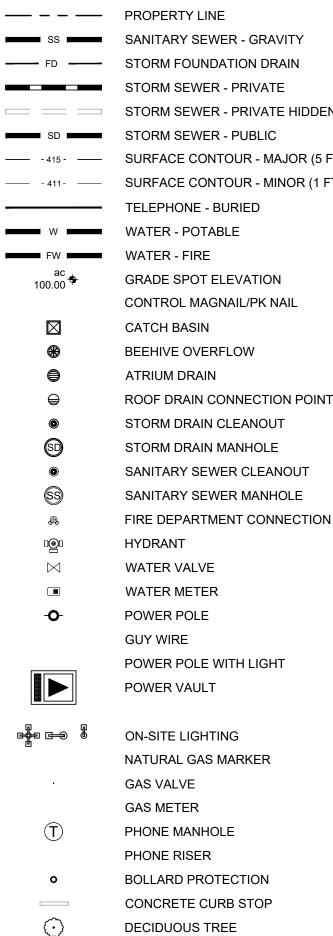
T/sub

TBC

TOS

DEMO	EXISTING	NEW	E LEGEND LISTED BELOW
DEMIC	EXIGNIC	4 Å	CONCRETE
			DEFERRED PAVING
			GRAVEL PAVING
			HMAC PAVING - STANDARD
			HMAC PAVING - HEAVY
		* * * *	LANDSCAPING
			RIP RAP
		M MM MM MM M	
			STORMWATER POND
		· ·	CONVEYANCE SWALE
			EASEMENT - PER PLAN
	X	— × —	FENCING
			FIBER
	——— UTL ———	——— UTL ———	DATA, LOW VOLTAGE
			GRADE BREAK
			IRRIGATION
	—— G ——	GAS —	NATURAL GAS / PROPANE
	—— Р ——	P —	POWER - BURIED
		——— OHP ———	POWER - OVERHEAD PROPERTY LINE
		SS SS	SANITARY SEWER - GRAVITY STORM FOUNDATION DRAIN
		—— FD ——	STORM FOUNDATION DRAIN STORM SEWER - PRIVATE
	—— D ——		
		SD ====	STORM SEWER - PRIVATE HIDDEN STORM SEWER - PUBLIC
	-415-		SURFACE CONTOUR - MAJOR (5 FT)
	-411-		SURFACE CONTOUR - MINOR (1 FT)
			TELEPHONE - BURIED
		10/	
		W ====	WATER - POTABLE
		FW ac	WATER - FIRE GRADE SPOT ELEVATION

Ó—X



CONIFEROUS TREE

SIGN (SHAPE VARIES)

DIRECTIONAL ARROWS

HANDICAP PARKING SYMBOL

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2/3/21

Revisions

No. Description 1 Pre-Bid Revisions

Stamp EXPIRES: 6-30-21

CONFORM SET

4/2/21

Project Number

20335

Drawing Title

GENERAL NOTES



- 1. CLEARING AND GRUBBING SHALL BE DONE PER NOTES ON C0.10.
- SEE SEPARATE 1200C PLANS FOR ADDITIONAL EROSION CONTROL MEASURES AND IMPLEMENTATION.

SITE LEGEND

GRAVEL CONSTRUCTION ENTRANCE

LIMITS OF DEMOLITION/SAWCUT

- 1 REMOVE EXISTING SIDEWALK
- REMOVE EXISTING GAURDRAIL
- REMOVE EXISTING FENCING
- REMOVE EXISTING TREE
- SAWCUT AND REMOVE EXISTING PAVEMENT
- REMOVE EXISTING CURB AND GUTTER
- EROSION CONTROL SILT FENCE
- CONSTRUCTION ENTRANCE
- PROTECT EXISTING SIGN
- 10 PROTECT EXISTING STORM INLET
- 11 PROTECT EXISTING POLE AND GUY WIRES

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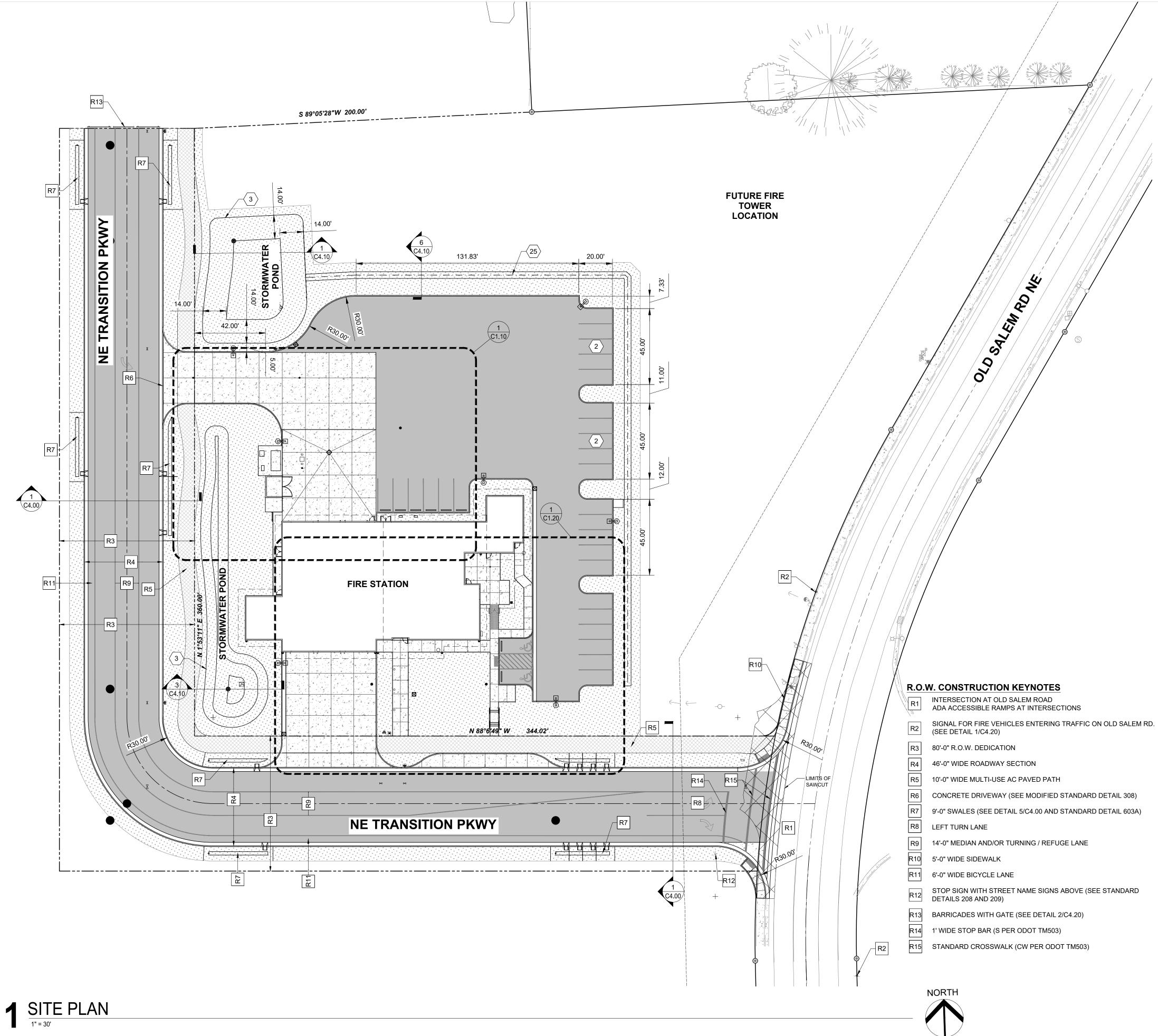
4/2/21

Project Number

20335

Drawing Title **EXISTING CONDITIONS AND DEMOLITION PLAN**

Sheet No **CO.20**



	GENERAL SITE INFORMATION		
	ITEM	DESCRIPTION	
	LOCATION	OLD SALEM ROAD - MILLERSBURG, OR	
	LEGAL DESCRIPTION	T 10 S R 03 W SEC 28	
	PROPERTY ID		
/	ZONING	PF (PUBLIC FACILITIES ZONE)	
	LOT SIZE	3.59 ac (PROPERTY LOT)	
/	IMPERVIOUS AREA	47,852 sf	
	PERVIOUS AREA	108,613 sf	

GENERAL BUILDING	INFORMAT	ION
BUILDING	SIZE	HEIGHT
STATION 15	9,935 SQFT	±22'-0"

PARKING REQUIREMENTS				
EMPLOYEES PER SHIFT			6	
TOTAL EMPLOYEES	S AT SHIFT CHANGE		12	
PARKING REQUIF	REMENTS PER MILLERSBUF	RG DEVELOPMENT CO	DE CH. 3.03	
VEHICLE SPACES	MIN. REQUIRED	RATIO	REQUIRED	
MUNICIPAL / GOVERNMENT	1 SP PER 2 EMPLOYEES + 1 SPACE PER 800 SQFT	12 EMPLOYEES	19	
TOTAL REQUIRED			19	
TOTAL PROVIDED			29	
BICYCLE STALL	MIN. REQUIRED	RATIO	REQUIRED	
	1 PER 20 VEHICLE SPACES		1	
TOTAL REQUIRED		1		
TOTAL PROVIDED			1	

NOTES

1. ALL PARKING LOT CURB RETURNS SHALL HAVE A 5'-0" RADIUS UNLESS NOTED OTEHRWISE.

SITE LEGEND

· · · · · · · · · · · · · · · · · · ·	CONCRETE PAVING - SEE DETAIL 3/C4.00
	HMAC PAVING - SEE DETAIL 4/C4.00
	HMAC PAVING - HEAVY - SEE DETAIL 2/C4.00
	LANDSCAPING - SEE LANDSCAPING PLANS
	RIP-RAP - SEE DETAIL 7/C4.00

SITE KEYNOTES

$\langle 1 \rangle$	FIRE STATION BUILDING, (SEE ARCH PLANS)

(2) EMPLOYEE AND COMMUNITY VISITOR PARKING AREA

STORMWATER POND (SEE GRADING AND LANDSCAPING PLANS)

ONSITE SIDEWALK (SEE DETAIL 4/C4.10)

TRASH ENCLOSURE (SEE ARCH PLANS)

6 NOT USED

4" WHITE PAVEMENT STRIPING

(8) EV CHARGING PEDESTAL (SEE ELECTRICAL PLANS)

√ 9
→ FLAG POLES (SEE ARCH AND LANDSCAPING PLANS)

GENERATOR AND CONCRETE PAD (SEE ELECTRICAL PLANS)

LIFE JACKET STORAGE AREA (SEE ARCH AND LANDSCAPING PLANS)

 $\langle 12 \rangle$ 6"x6'-0 CURBSTOP (TYP.)

MECHANICAL PAD (SEE ARCH AND MECH PLANS)

(14) PARKING SIGN (TYP.) (SEE ARCH PLANS)

(15) UNDERGROUND OIL/WATER SEPARATOR (SEE PLUMBING PLANS)

(16) WASH AREA CATCH BASIN (SEE C3.00)

\$\langle 17 \rangle STRAIGHT CURB (SEE STANDARD DETAIL 304)

(18) MULTI-USE PATH (SEE DETAIL 5/C4.00)

(19) PATIO AND FENCE (SEE ARCH AND LANDSCAPING PLANS)

TYPICAL CURB AND GUTTER WITH DEPRESSED CURB (SEE STANDARD

DETECTABLE WARNING (SEE ODOT STANDARD DETAIL RD902)

ADA PARKING SIGN AND STRIPING (SEE DETAIL 6/C4.10)

TYPICAL CURB AND GUTTER (SEE STANDARD DETAIL 304)

24 STEPS AND HANDRAILS (SEE STANDARD DETAILS RD120, RD770, RD771)

CONVEYANCE SWALE (SEE DETAIL 6/C4.10)

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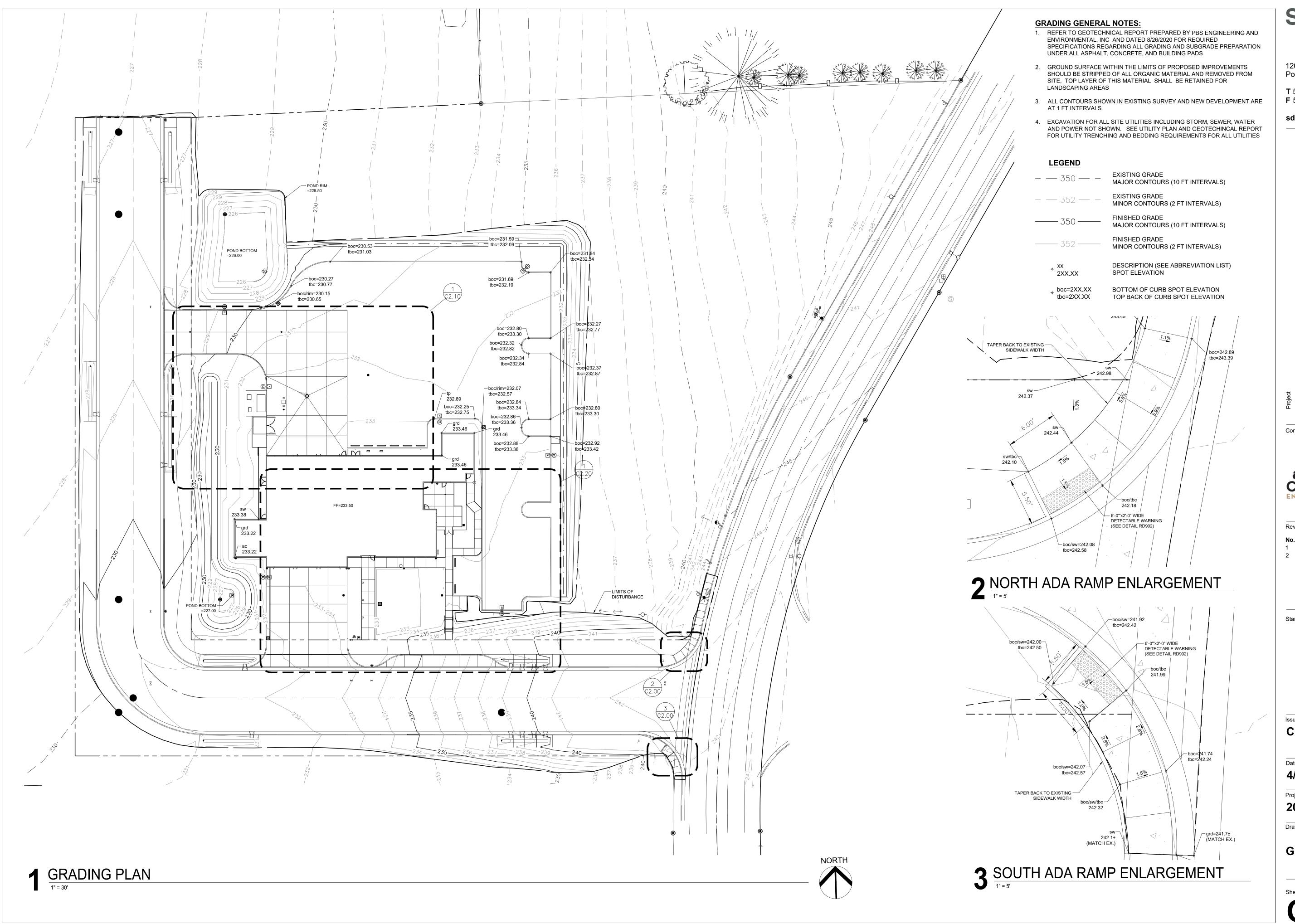
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Drawing Title

SITE PLAN



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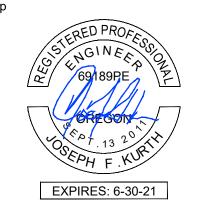
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3/2/21

Revisions

No. Description 1 Pre-Bid Revisions 2 Addendum 2



CONFORM SET

4/2/21

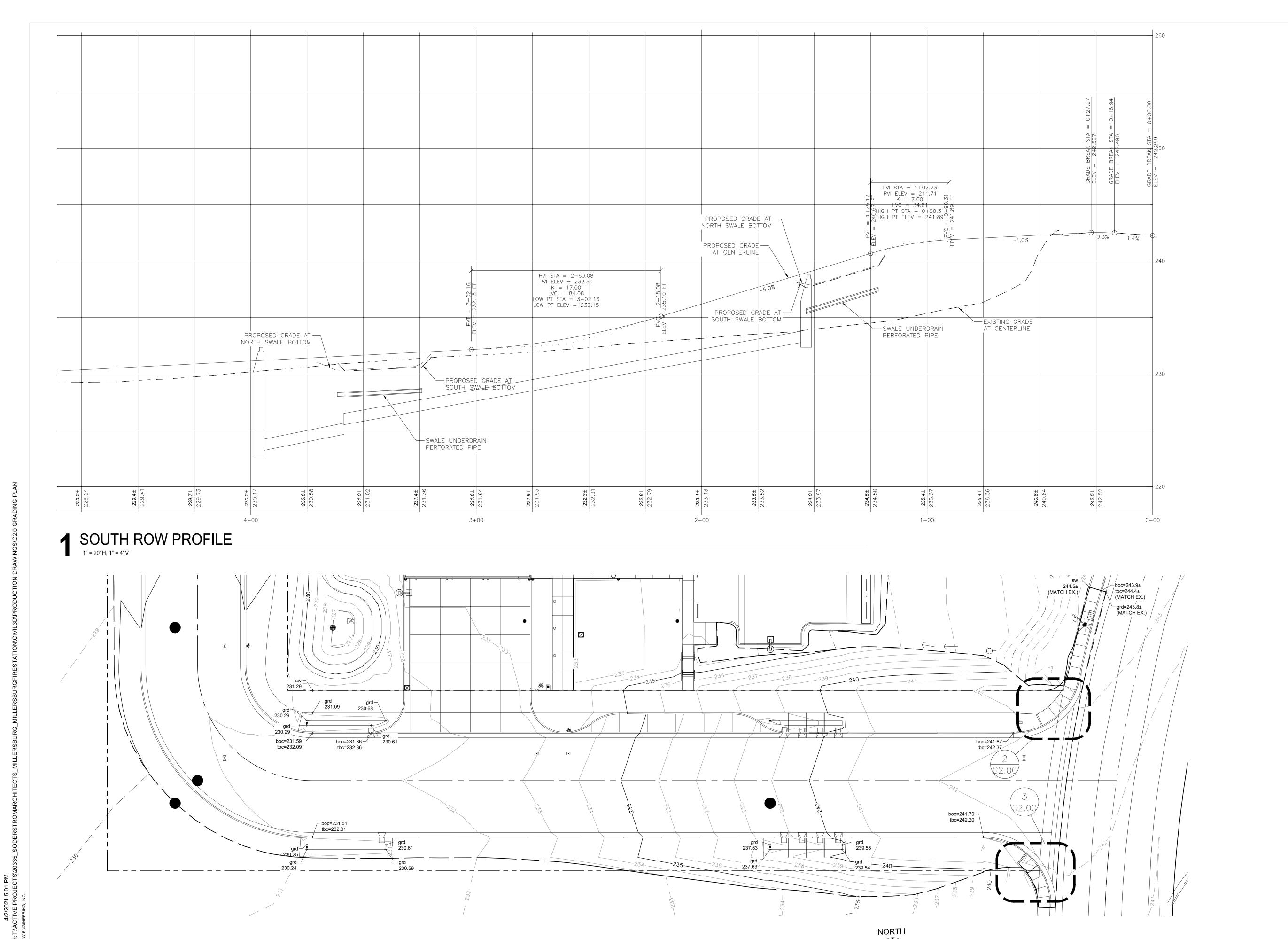
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Drawing Title

GRADING PLAN

C2.00



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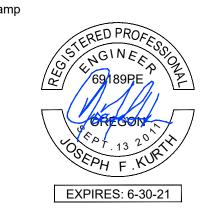
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ENGINEERING

F 503-213-2018

crowengineering.com

Revisions

1 Pre-Bid Revisions



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4/2/21

Project Number 20335

Drawing Title

SOUTH ROW GRADING PLAN

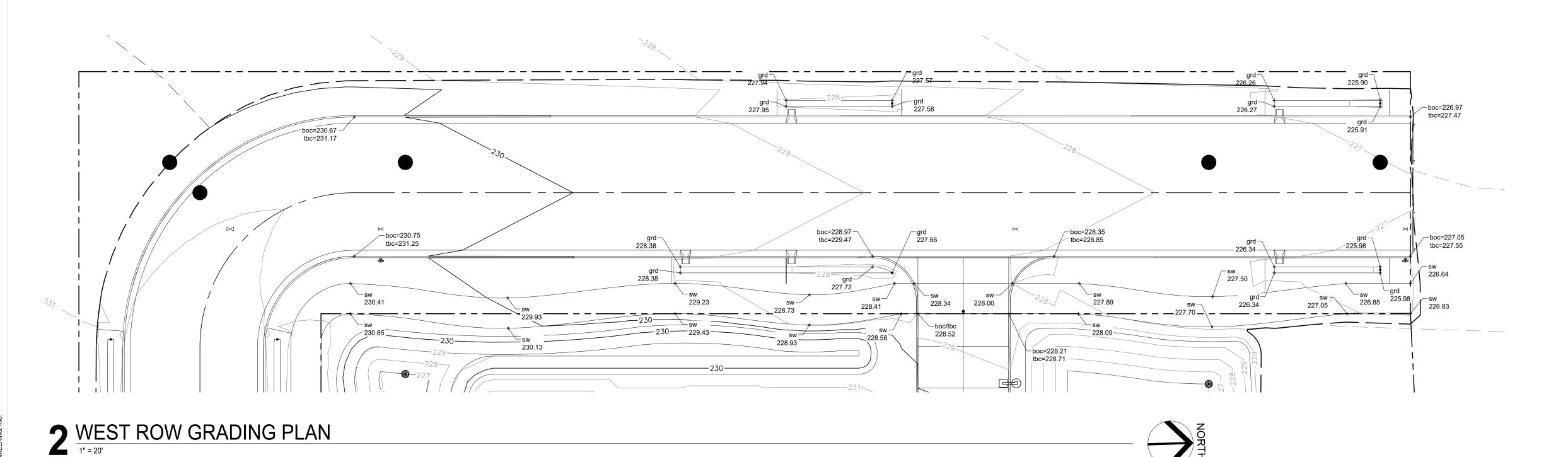
C2.30

2 SOUTH ROW GRADING PLAN

1" = 20'

WEST ROW PROFILE

1" = 20' H, 1" = 4' V



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Revisions

No. Description1 Pre-Bid Revisions 2 Addendum 2

CONFORM SET

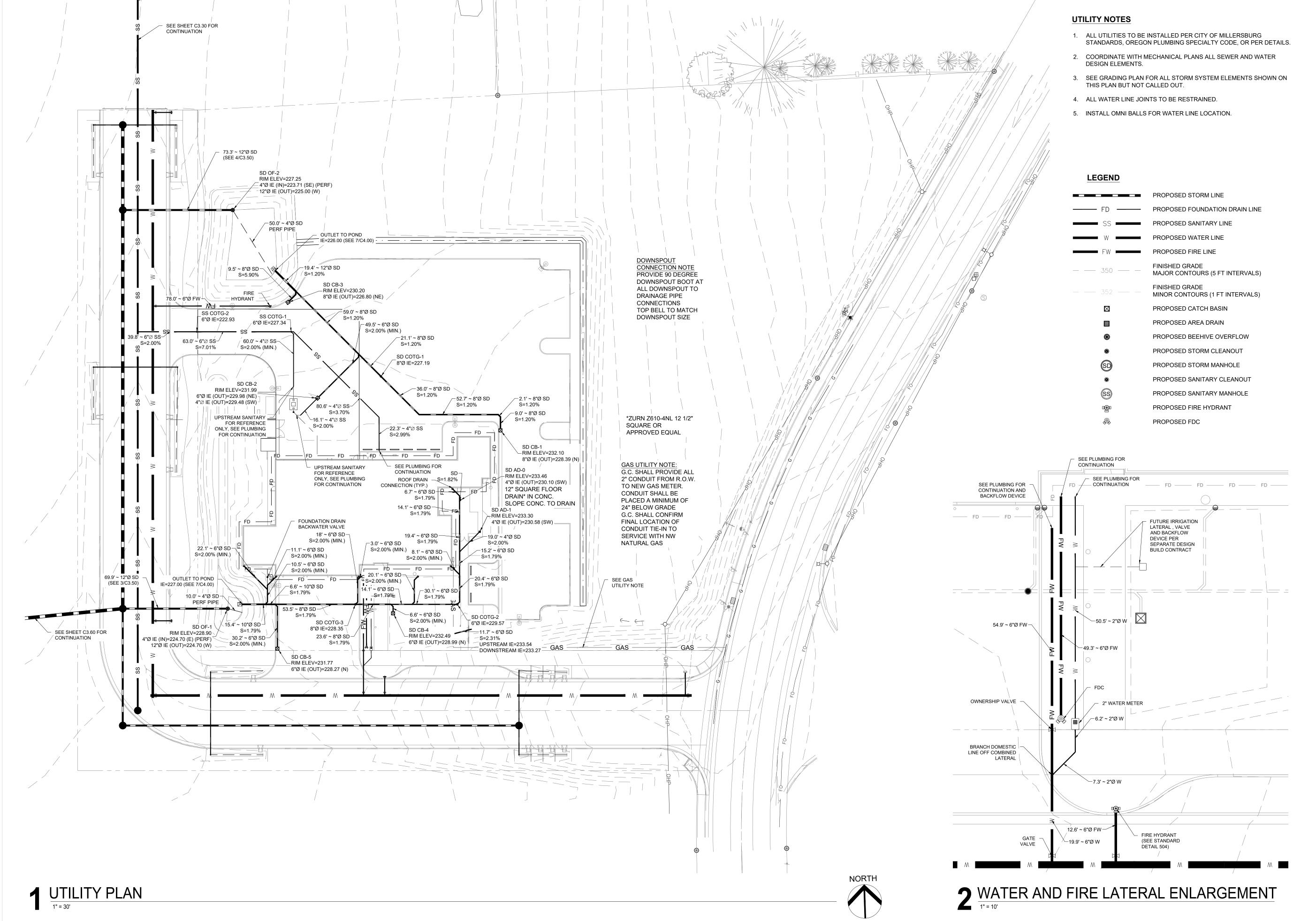
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Drawing Title

WEST ROW GRADING PLAN

Sheet No **C2.40**



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Revisions

2/3/21 1 Pre-Bid Revisions 3/2/21 2 Addendum 2



CONFORM SET

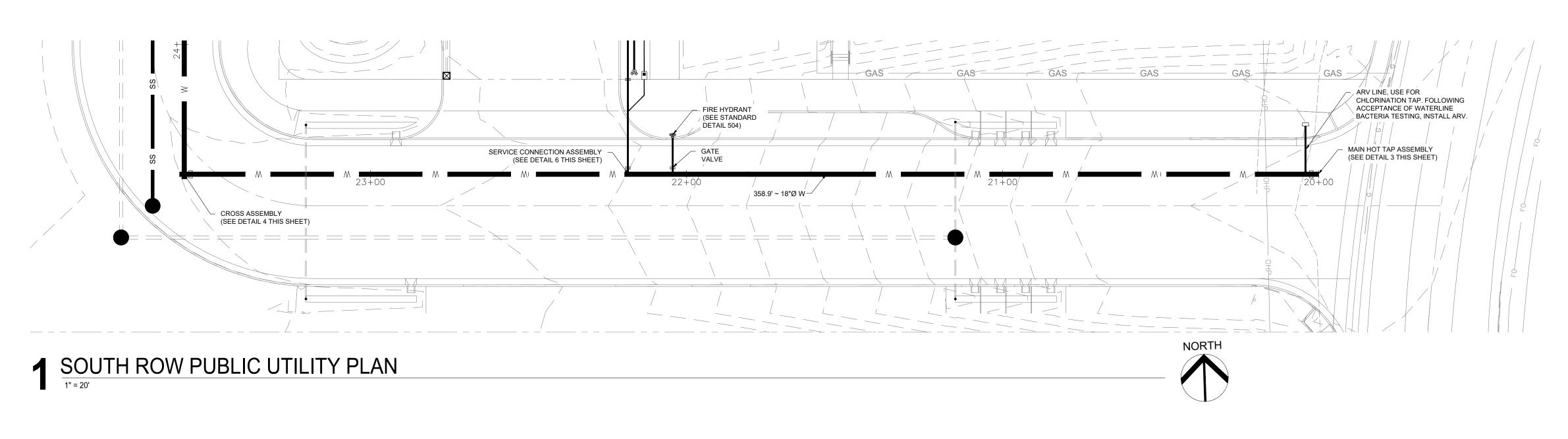
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Project Number 20335

Drawing Title

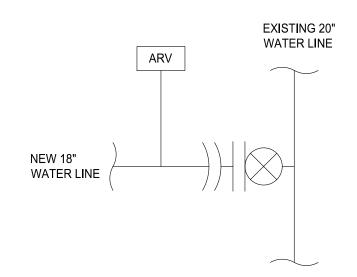
SITE UTILITY PLAN

C3.00



STA:20+04.17 ARV LINE -CONNECTION PROPOSED GRADE ABOVE PROPOSED WATER LINE STORM LATERAL STA:22+18.54 SERVICE LATERAL-STA:23+58.88 CONNECTION CROSS ASSEMBLY STA:20+00.00 (SEE DETAIL 4 THIS SHEET) MAIN HOT TAP ASSEMBLY -(SEE DETAIL 3 THIS SHEET) ─358.9' ~ 18"Ø W STA:22+04.41 STORM LATERAL CROSSING 23+65 23+00 22+00 21+00 20+**09**+95

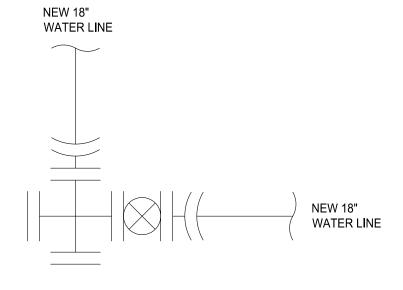
2 SOUTH ROW WATER MAIN PROFILE 1" = 20' H, 1" = 4' V



ASSEMBLY COMPONENTS

- (1) 18" FLG x MJ ADAPTER WITH RETAINER GLAND
- (1) ARV ASSEMBLY PER STANDARD DETAIL 509

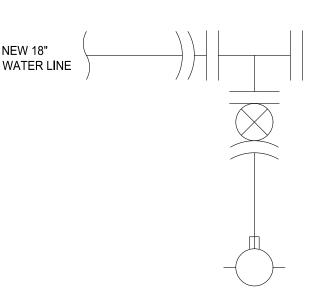
3 MAIN HOT TAP ASSEMBLY



ASSEMBLY COMPONENTS

- (1) 18" x 18" ALL FLG CROSS
- (2) 18" BLIND FLANGE (W&S)
- (1) 18" FLG BV
- (2) 18" FLG x MJ ADAPTER WITH RETAINER GLAND

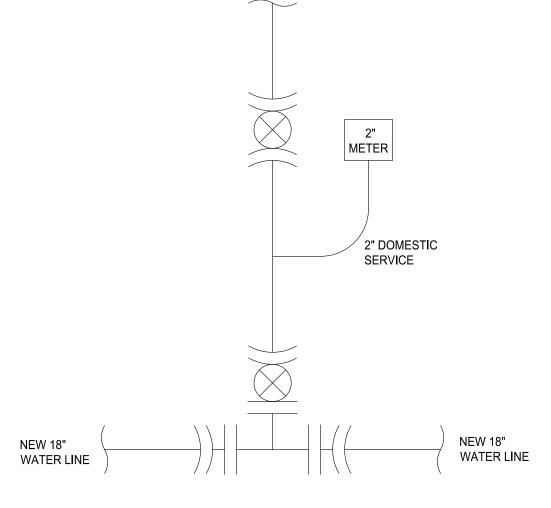




ASSEMBLY COMPONENTS

- (1) 18" x 6" ALL FLG TEE
- (1) 18" BLIND FLANGE
- (1) 18" FLG X MJ ADAPTER WITH RETAINER GLAND
- (1) FIRE HYDRANT ASSEMBLY PER STANDARD DETAIL 504

5 END OF LINE ASSEMBLY



FIRE LINE

ASSEMBLY COMPONENTS

- (1) 18" x 6" ALL FLG TEE
- (1) 6" FLG x MJ GV WITH RETAINER GLAND
- (2) 18" FLG x MJ ADAPTORS WITH RETAINER GLAND
- (1) 6" MJ x MJ GV WITH 2 RETAINER GLANDS

6 SERVICE CONNECTION ASSEMBLY

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EXPIRES: 6-30-21 **CONFORM SET**

4/2/21

Project Number

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Drawing Title **SOUTH ROW UTILITY PLAN AND PROFILE**

C3.10



- 1. FOR WATER/SANITARY CROSSING WITH LESS THAN 18" OF VERTICAL SEPARATION, INSTALL CLSM
- SUPPORT PER DETAIL 3 THIS SHEET. 2. BLOW OFF WATERLINE THROUGH 6" LINE PRIOR TO INSTALLING HYDRANT, THEN INSTALL HYDRANT PRIOR TO PRESSURE TESTING.

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Revisions

1 Pre-Bid Revisions

Date 2/3/21



CONFORM SET

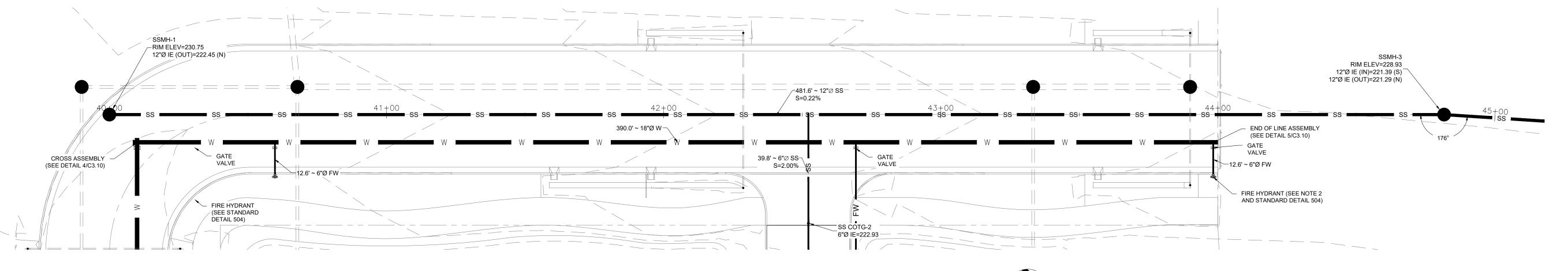
4/2/21

Project Number 20335

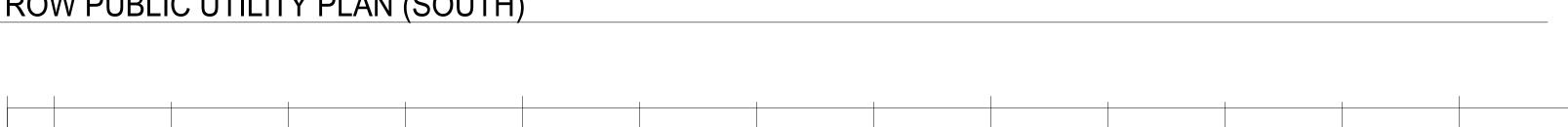
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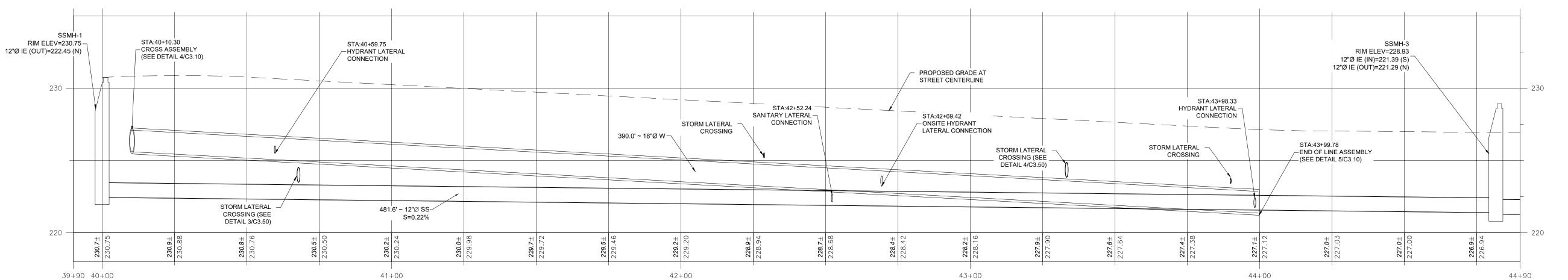
WEST ROW UTILITY PLAN AND PROFILE

C3.20



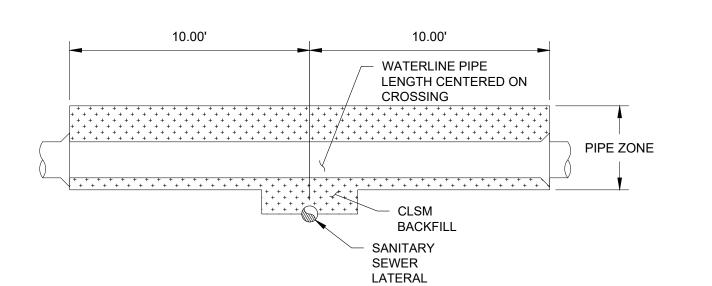
WEST ROW PUBLIC UTILITY PLAN (SOUTH)



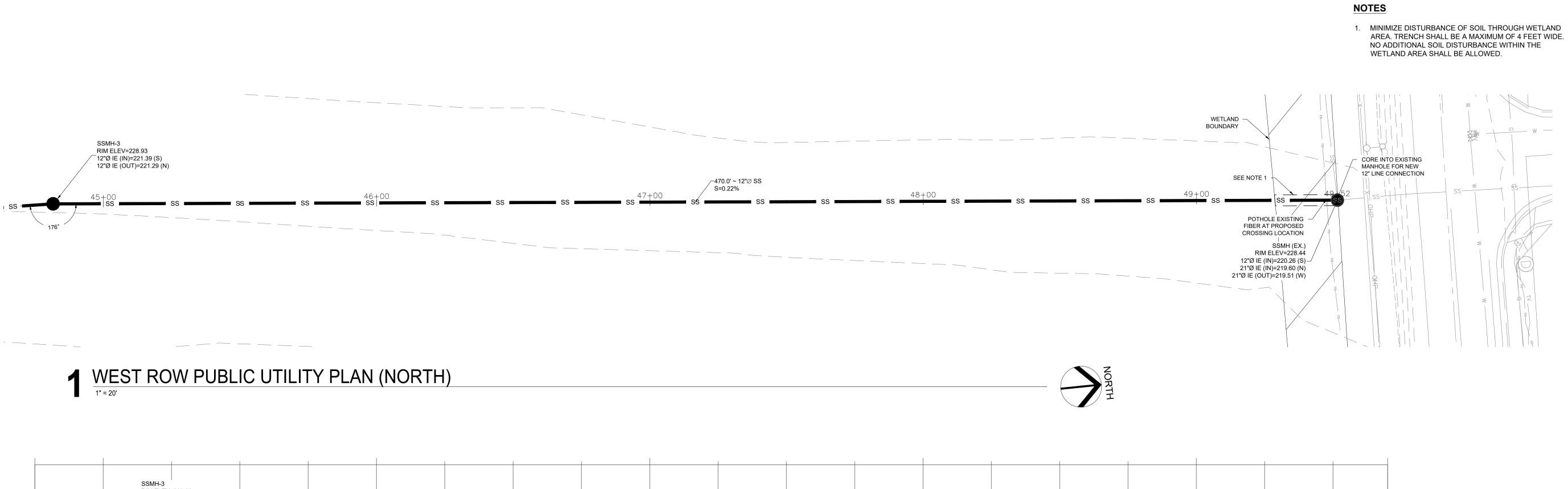


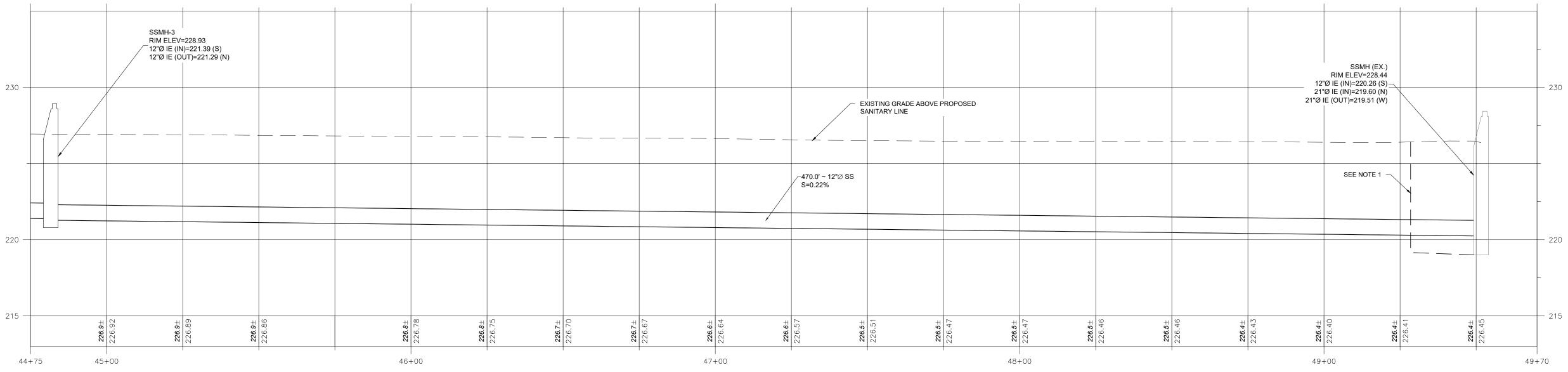
2 WEST ROW PUBLIC UTILITY PROFILE (SOUTH)

1" = 20' H, 1" = 4' V



3 WATER/SANITARY CLSM BACKFILL





2 WEST ROW PUBLIC UTILITY PROFILE (NORTH)

1" = 20' H, 1" = 4' V

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NORTH SANITARY **SEWER PLAN AND PROFILE**

C3.30

ROW STORM NOTES

SD SW-OF-1 -RIM ELEV=237.98 4"Ø IE=235.44

8.0' (TYP.)

36.5' ~ 4"Ø SD —

19.5' ~ 4"Ø SD

SD SW-OF-2

~RIM ELEV=237.88 = ✓ 4"Ø IE=235.34 -

32.0' —

SD MH-5 RIM ELEV=238.76 —4"Ø IE (IN)=232.95 (N)

4"Ø IE (IN)=232.95 (S)

12"Ø IE (OUT)=232.85 (W)

CHECK DAM (TYP.) SEE STANDARD DETAILS 603A AND 618 — SLOPE ALL ROADSIDE SWALE OVERFLOW LATERALS AS NECESSARY TO AVOID CONFLICTS WITH

CROSSING UTILITIES. MAINTAIN A MINIMUM 2% SLOPE.

SOUTH ROW STORM PLAN

77.9' ~ 12"Ø SD-

SD MH-4 RIM ELEV=232.35 12"Ø IE (IN)=223.39 (E)

12"Ø IE (OÙT)=223.29 (N)

S=0.50%

SD SW-OF-3

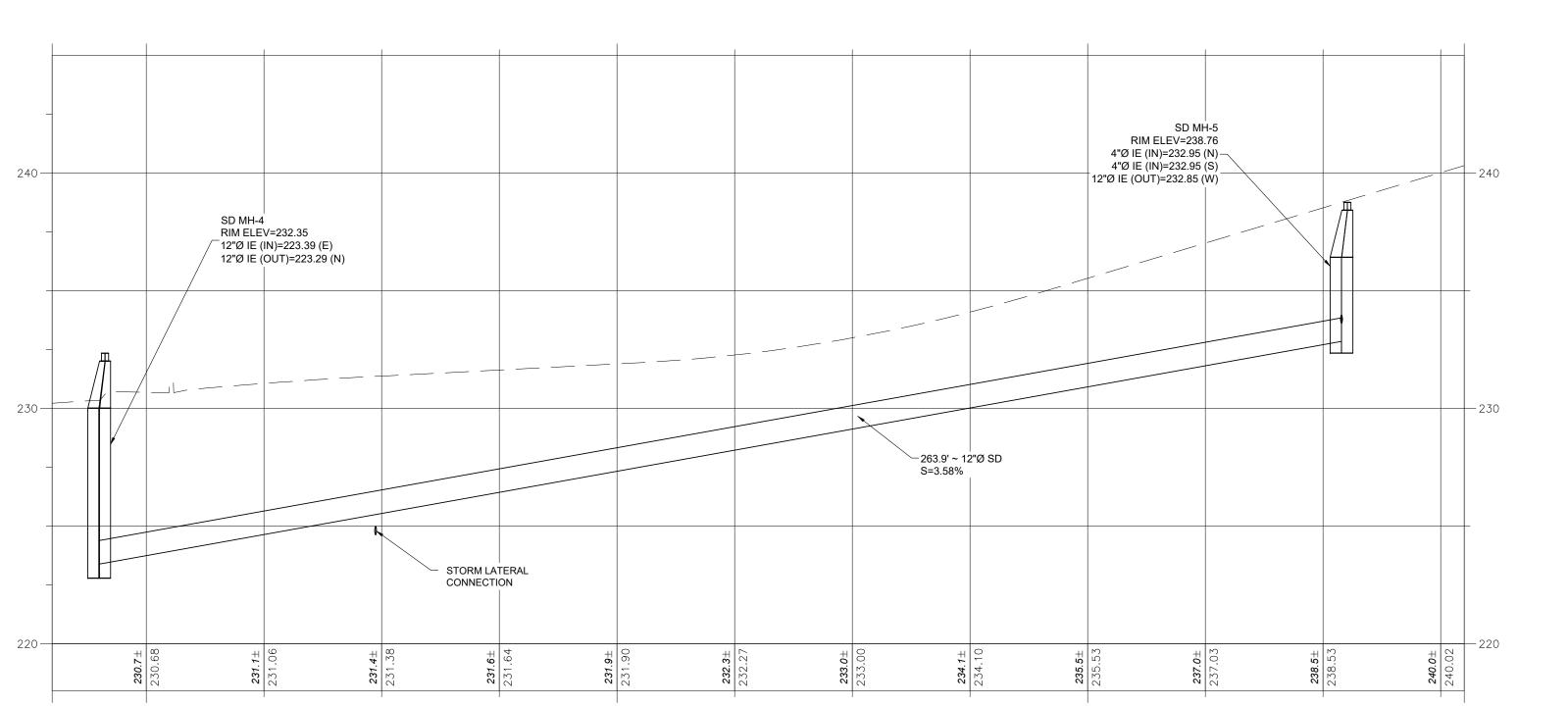
4"Ø IE=227.99

─35.6' ~ 4"Ø SD

SD SW-OF-4 RIM ELEV=230.49-RIM ELEV=230.54

35.0'

- SHALLOW SWALE (TYP.), SEE DETAIL 5/C4.00 AND STANDARD DETAILS 603A AND 603B



2 SOUTH ROW STORM PROFILE

1" = 20' H, 1" = 4' V

14+20

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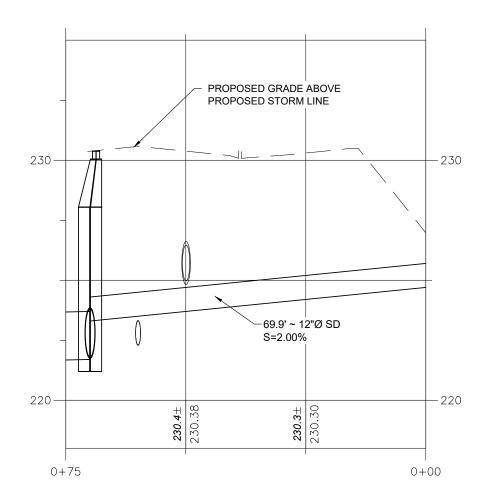
Drawing Title

SOUTH ROW STORM PLAN AND PROFILE

Sheet No **C3.40**

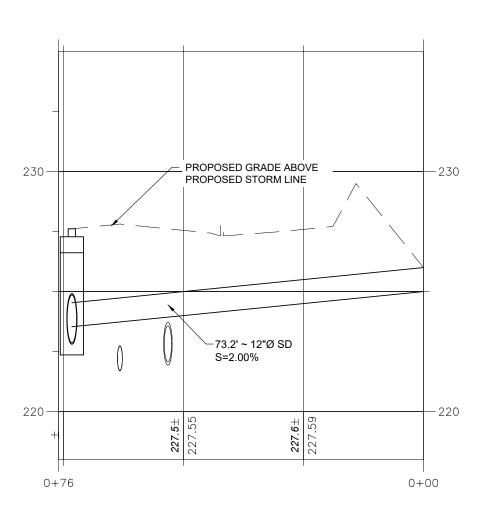
2 WEST ROW STORM PROFILE 1" = 20' H, 1" = 4' V

64+10 64+00



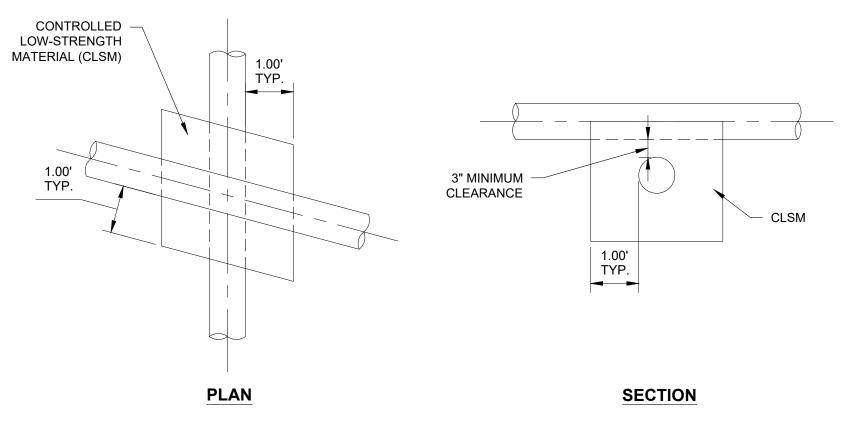
3 SOUTH OVERFLOW LATERAL PROFILE

1" = 20' H, 1" = 4' V



4 NORTH OVERFLOW LATERAL PROFILE

1" = 20' H, 1" = 4' V



60+00 59+90

61 + 00

1. FOR CLSM REQUIREMENTS, REFER TO THE OREGON DOT STANDARD SPECIFICATION FOR CONSTRUCTION, SECTION 00442, 2021 EDITION.

5 CLSM SUPPORT

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No. Description 1 Pre-Bid Revisions 2 Addendum 2



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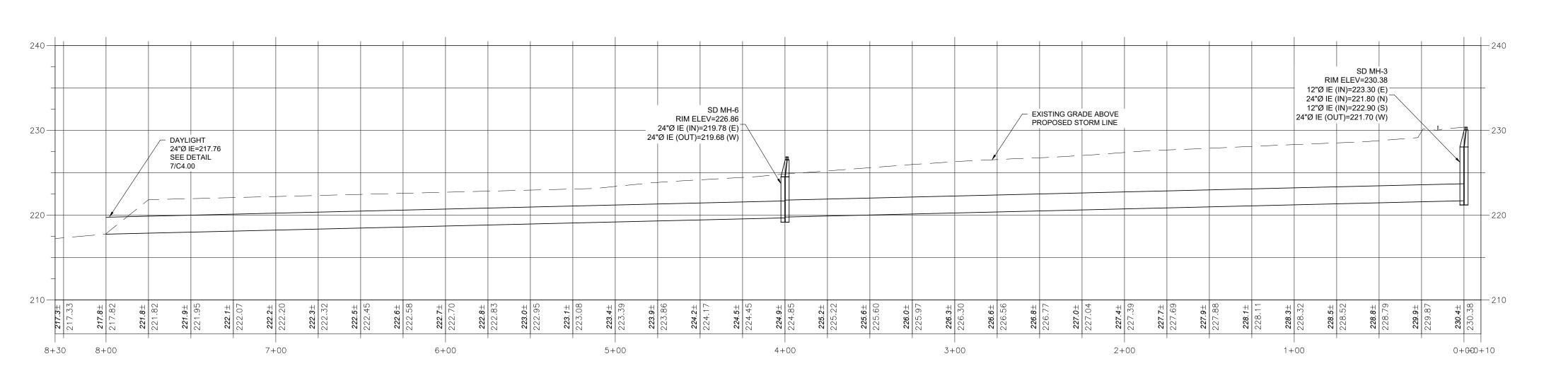
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Drawing Title

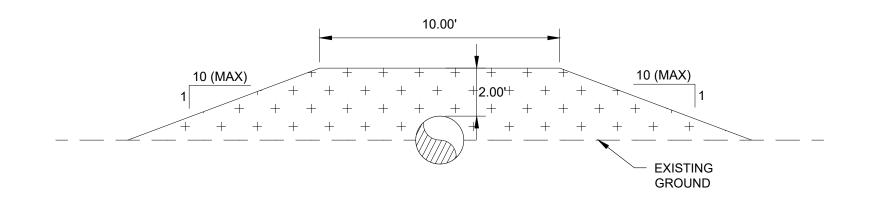
WEST ROW STORM PLAN AND PROFILE

C3.50



2 WEST STORM OUTFALL PROFILE

1" = 40' H, 1" = 8' V



3 OUTFALL PIPE COVER DETAIL

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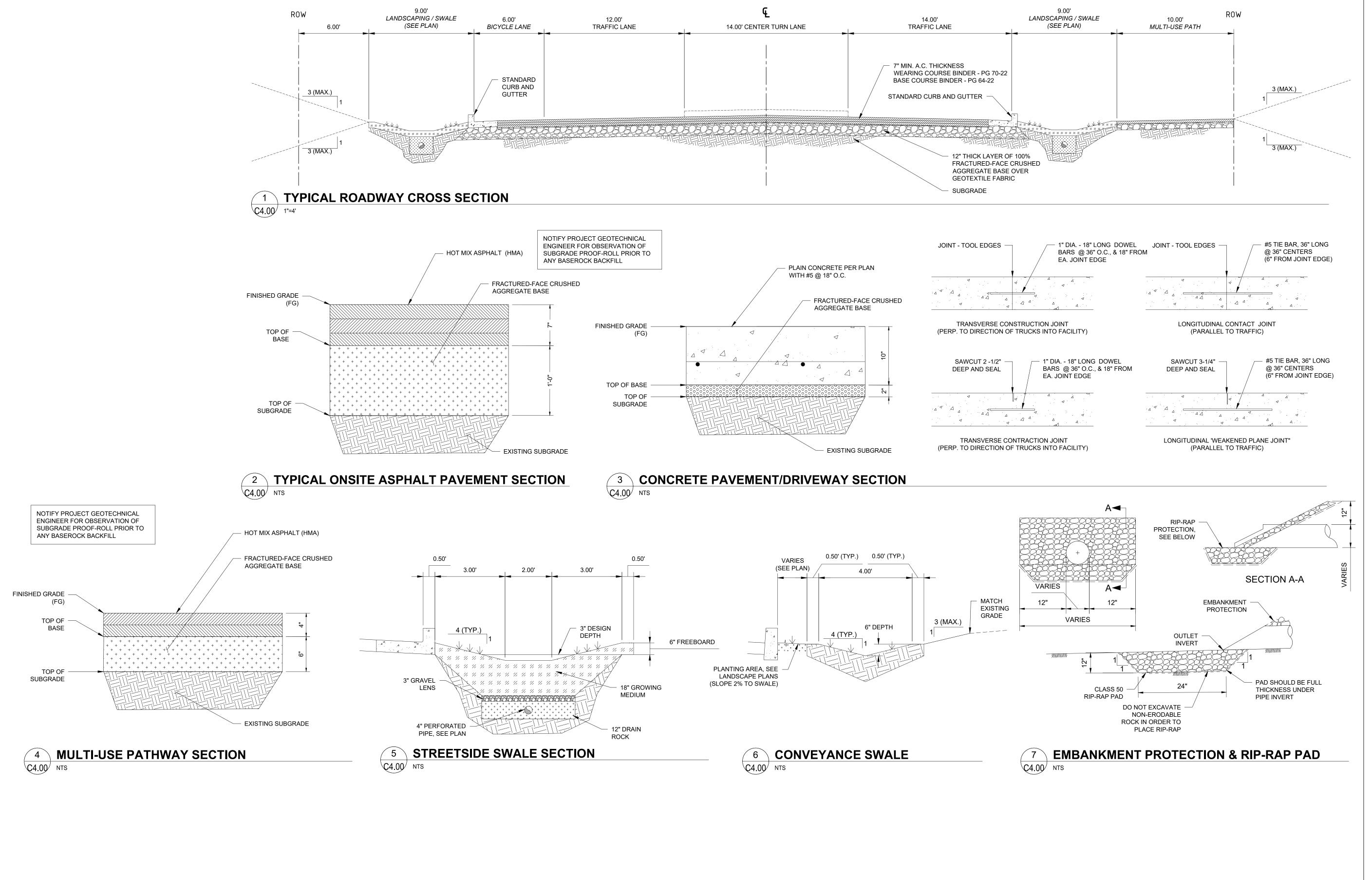
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Project Number

20335

Drawing Title **WEST STORM OUTFALL PLAN AND PROFILE**

C3.60



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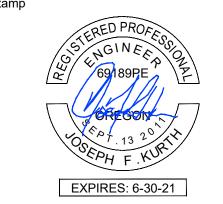


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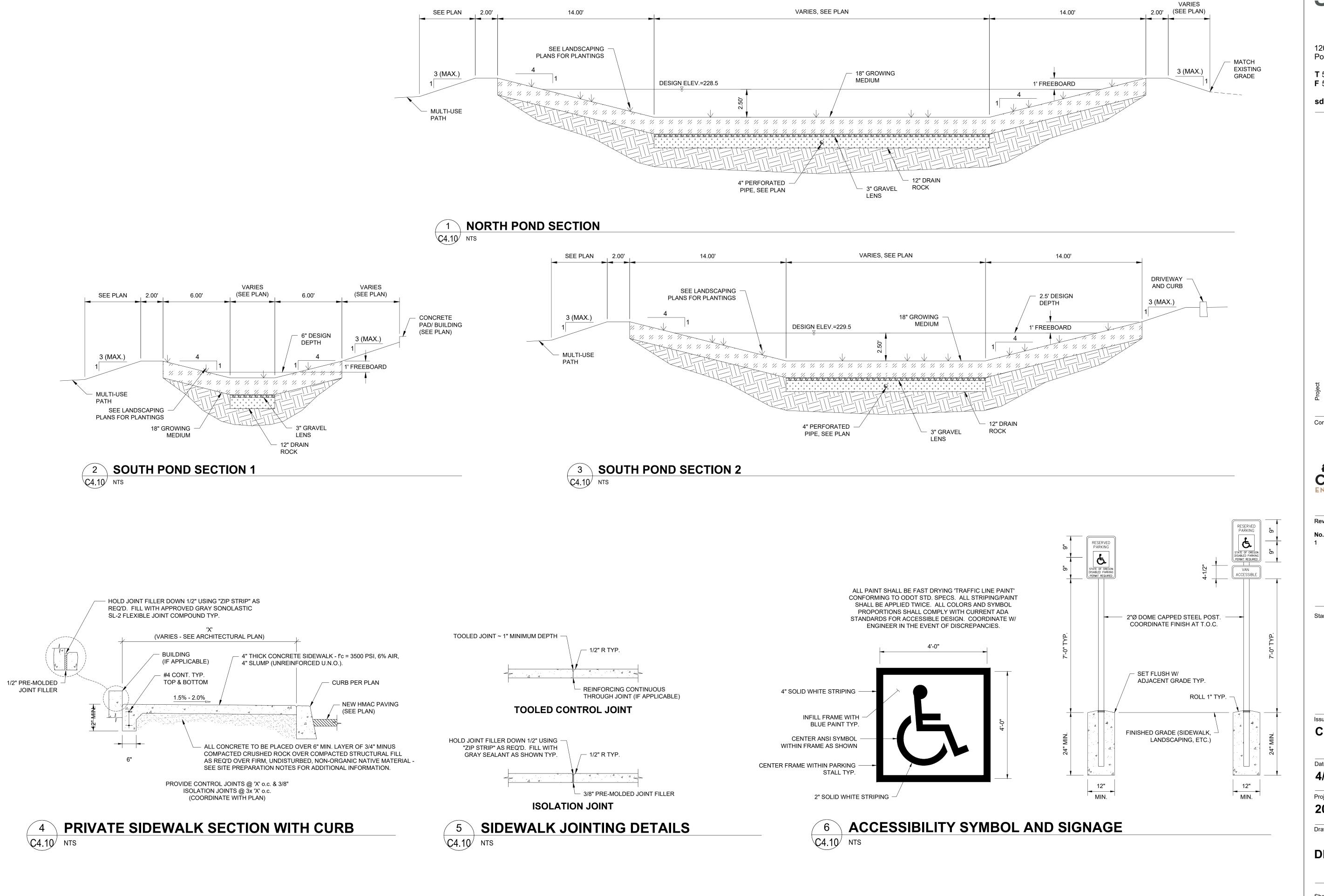
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Drawing Title

DETAILS

C4.00



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T 503-228-5617 **F** 503-227-8584

sdra.com

Transition Millersburg Station



Consultant



Revisions

No. Description 1 Pre-Bid Revisions

EXPIRES: 6-30-21

CONFORM SET

4/2/21

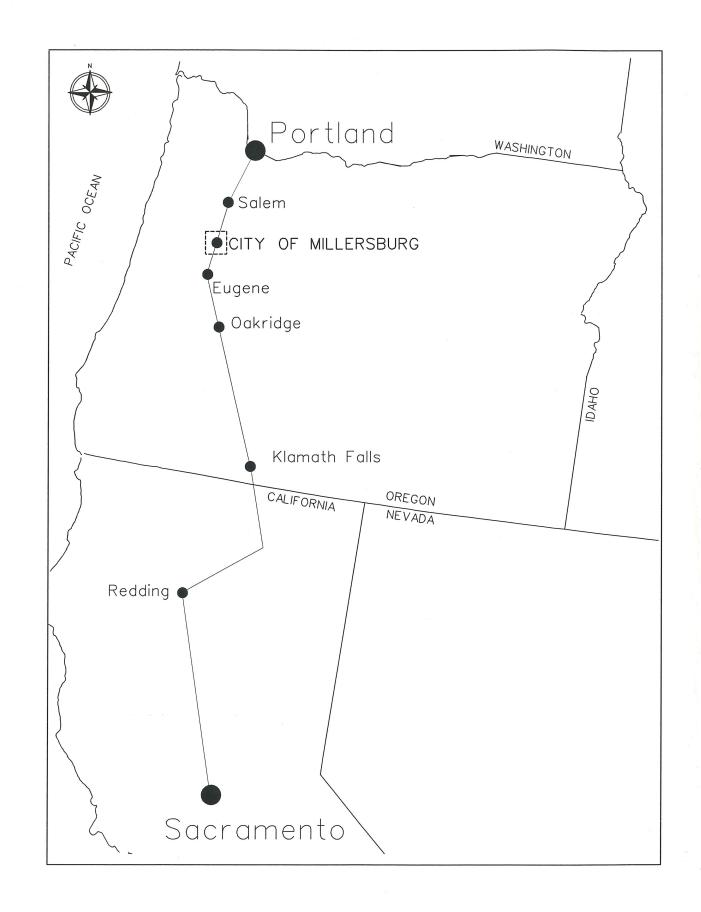
Project Number

20335

Drawing Title

DETAILS

8. Level 3 Communications Fiber-Optic Cable Route



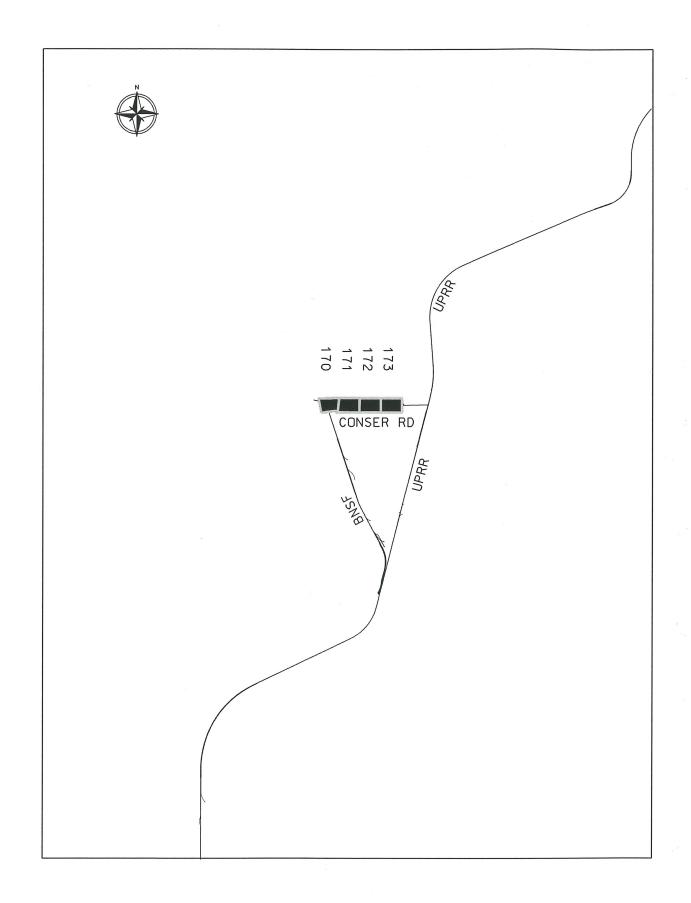


FIBER-OPTIC CABLE ROUTE

CONSTRUCTION DRAWINGS

FOR LEVEL 3 COMMUNICATIONS

PROJECT SPAN
CITY OF MILLERSBURG





CITY OF MILLERSBURG INDEX

DRAWING	NO.	STREET NAME
	170	Woods Roads
	171	Waverly Drive
		Amenda Lane
	172	Bain Street
	173	Approach Road

As-Built Annotation

Description	Options
Duct Type	H = HDPE P = PVC Z = Other
Duct Bank Count	2, 3, 4, 6, 8, 10, 12, 14, 16, 18, 20, 22, 24, 26, 28, 30 Z = Other
Installation Method (other than plow or trench)	A = Attachment (i.e. bridges) B = Bore Z = Other No annotation if duct is plowed/trenched
Duct Bank Casing Type	F = FRE casing H = HDPE casing P = PVC casing S = Steel casing Z = Other
Duct Bank Casing Size	8 = 8 inch casing 9 = 9 inch casing 10 = 10 inch casing 12 = 12 inch casing 2*5 = 2-5 inch casing 2*6 = 2-6 inch casing Z = 0+her
Duct Bank Encasement	C = Concrete 0 = Flowable fill M = Multicell (integrated duct and casing system) T = Steel plate on top of duct bank Z = Other
Duct Bank Cover Depth	Inches 20 (20" or less) 20/40 40/60 60/100 100 (100" or more)
Surface Restoration	PR = Pavement Restoration Roadway PW = Pavement Restoration Walkway
Handho I e	HHA = FRP: API Model: 13-4800-42, concrete cover HHB = FRP: API Model: 13-4800-42, cast iron cover HHZ = Other
Manho I e	MHA = FRP: API Model 13-4882-20, hatch doors MHB = FRP: API Model 13-4882-45, cast iron cover MHC = Manhole, 4.0' x 4.0' x 4.0' MHZ = Manhole (other)

The locations of utilities shown on this drawing are only approximate. Level 3 Communications, LLC hereby disclaims any responsibility to third parties for the accuracy of this information. Persons working in the area covered by this drawing must contact the statewide Call-Before-You-Dig System to ascertain the location of underground utilities prior to performing any excavation.

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CALL BEFORE YOU DIG 1-800-332-2344

48 HOURS NOTICE REQUIRED

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Kiewit Network

PARSONS BRINCKERHOFF

<	Services	Co.

Program: Intercity Network
City Pair: Sacramento to Portland

Package Title: City of Millersburg

Drawing Title: As-Built Annotation Code Legend

City Pair Code Span No. Package No. Type Drawing No. Rev SACPOR 10 Ρ L1 Α

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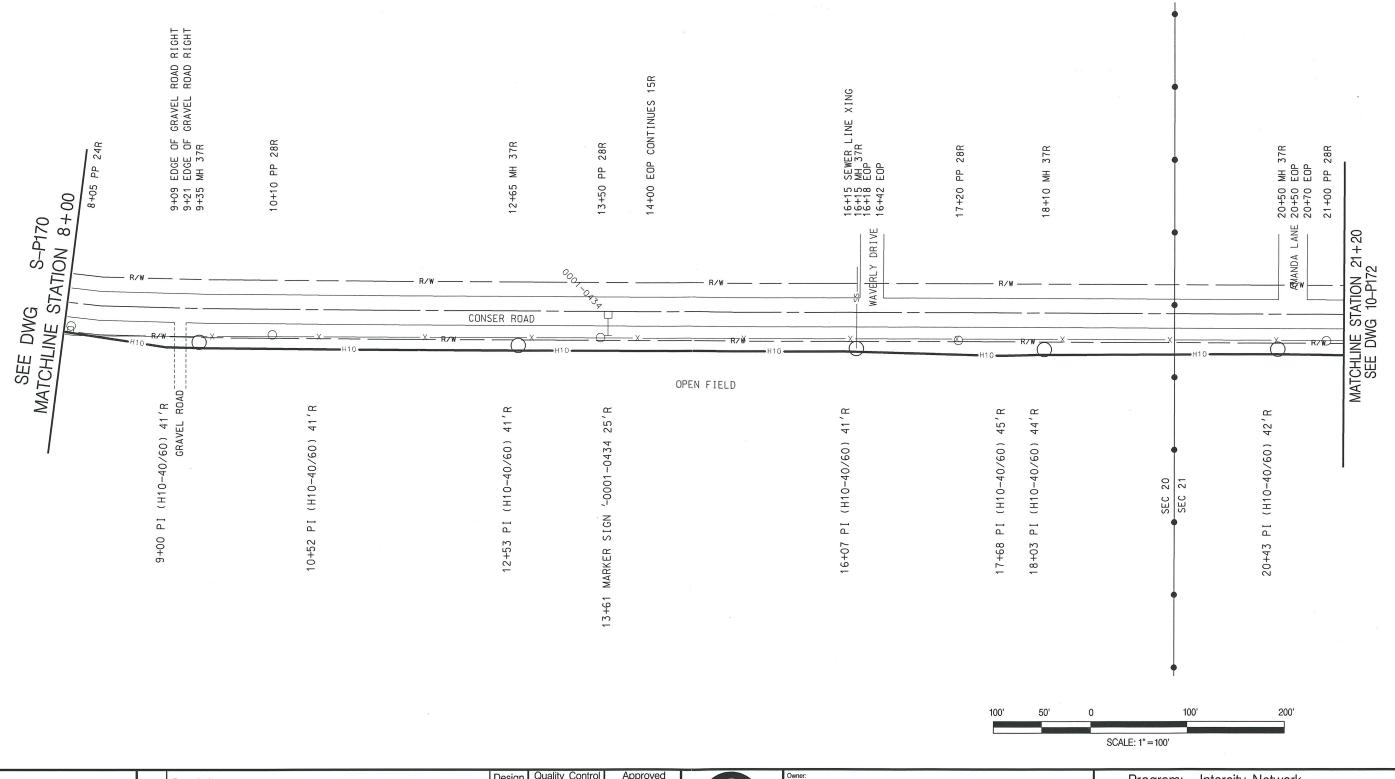
Program:	Intercity Network
City Pair:	Sacramento to Portland
Package Title:	City of Millersburg
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Drawing Title: 28+06 to 8+00

City Pair Code	Span No.	Package No.	Type	Drawing No.	Rev	Sheet No.
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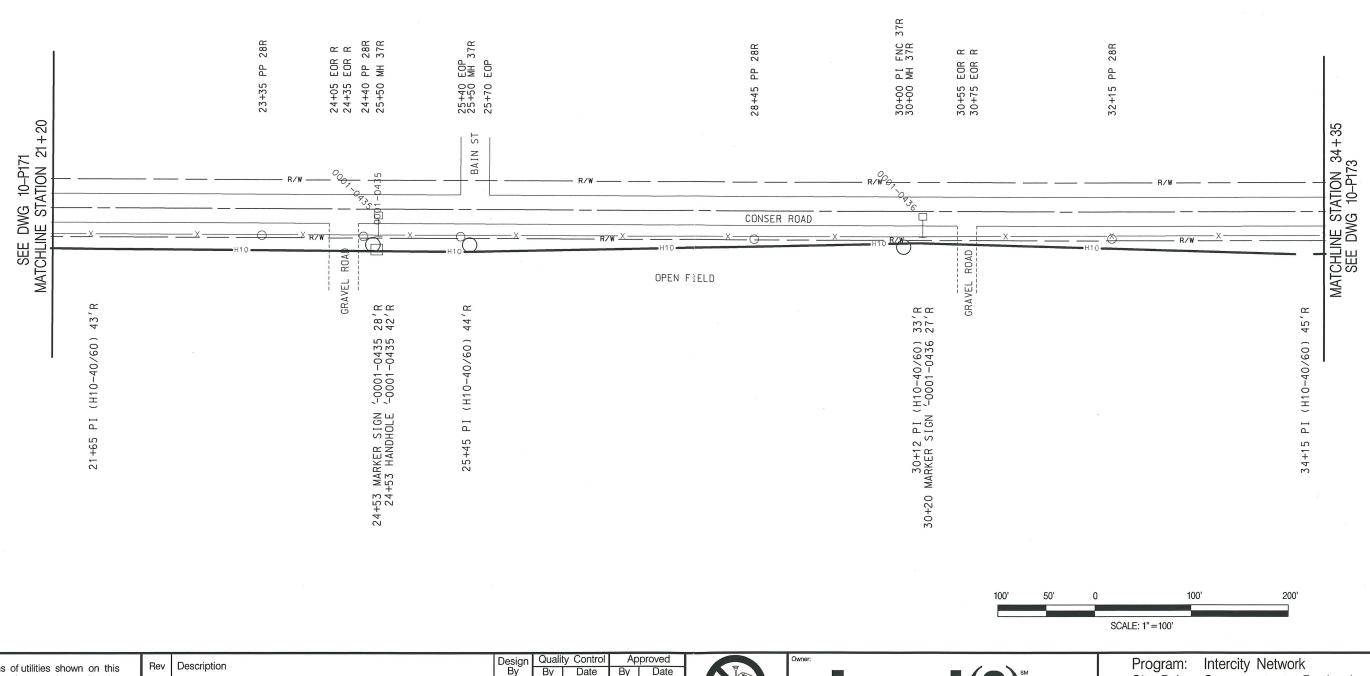
Miewit Network Services Co. PARSONS BRINCKERHOFF

Intercity Network City Pair: Sacramento to Po Package Title: City of Millersburg Sacramento to Portland

Drawing Title: 8+00 to 21+20

City Pair Code	Span No.	Package No.	Туре	Drawing No.	Rev	Sheet No.
SACPOR	10		Р	171	Α	





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The locations of utilities shown on this

Rev Description

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CALL BEFORE YOU DIG 1-800-332-2344 48 HOURS NOTICE

REQUIRED

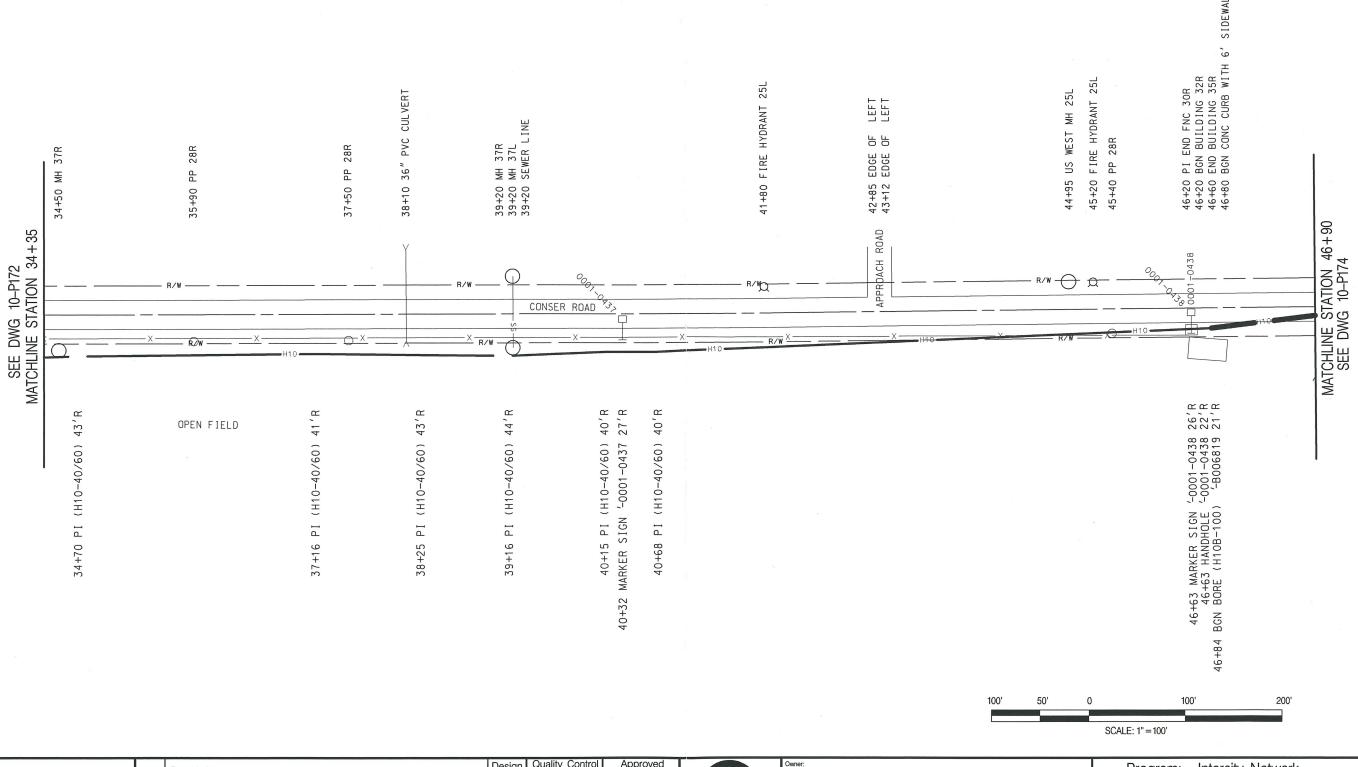
Micwit Network Services Co. PARSONS BRINCKERHOFF

City Pair: Package Title: City of Millersburg Drawing Title: 21 + 20 to 34 + 35

City Pair Code	Span No.	Package No.	Type	Drawing No.	Rev	Sheet No.
SACPOR	10		Р	172	Α	

Sacramento to Portland





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ALL BEFORE YOU DIG 1-800-332-2344

48 HOURS NOTICE

REQUIRED

Level	(3)
COMMINICATIONS	

Michigan Kiewit Network Services Co. PARSONS BRINCKERHOFF

Program:	Intercity Network
City Pair:	Sacramento to Portland
Packagé Title:	City of Millersburg
Drawing Title:	34 + 35 to $46 + 90$

City Pair Code Span No. Package No. Type Drawing No. Rev Ρ SACPOR 10 173 A

Sheet No.